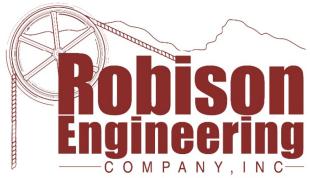
### Sky Tavern Junior Ski Area Expansion SPECIAL USE PERMIT

#### Prepared for:

Sky Tavern at 21130 Mt. Rose Highway Reno, NV 89511

#### Prepared By:



846 Victorian Ave., Suite 20 Sparks, NV 89431 (775) 852-2251

September 2023



#### **Submitted to:**

Washoe County, Nevada

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Washoe County, Nevada

#### PROPERTY LOCATION

Sky Tavern Junior Ski Area is a destination ski resort that is located in the southwest corner of Washoe County approximately 10 miles north of Carson City and 10 miles south of Reno. The ski resort is accessed via Nevada State Route 431 also known as Mt. Rose Highway. The primary access to the ski resort is a parking lot at the east side of the property which can be accessed from Mt. Rose Highway or Bum's Gulch Rd. The ski resort is contained within one PR (parks and recreation) zoned property with Washoe County Assessor's Parcel Number 048-050-03 owned by the City of Reno and with an acreage of 143.07.

#### **SPECIAL USE PERMIT REQUESTS**

A special use permit is requested for the following components:

- Allowance of a utility service within a PR zone, including 1-million & 2-million gallon snowmaking water storage tanks, approximately 11,000 linear feet of snowmaking water supply piping, and two groundwater wells to supply the system each equipped with pump assemblies enclosed in pump house structures. Electrical to be supplied from existing on-site NVE facilities and service lines will be joint-trenched with water wherever possible.
- Allowance of site lighting within a PR zone
- Allowance to vary the landscaping standards per Washoe County Development Code section 110.412.50(a) – At least one tree shall be provided for every 10 parking spaces...
  - Due to the nature of this facility, landscaping islands or trees in the parking area are impractical and would not achieve the code's intent of screening, shade, etc. The entire area is forested, and winter parking is generally on top of accumulated snow which must be aggressively managed by heavy equipment; isolated trees and curb/gutter infrastructure would be damaged or destroyed.
- Allowances for grading associated with Washoe County Grading code which require a special use permit including:
  - 110.438.35(a)(2)(ii)(A) Grading on slopes 15% or greater excavation of one thousand (1,000) cubic yards or more whether the material intended to be permanently located on the project site or temporarily stored on a site for relocation to another, final site
    - Approximately 5,000 cubic yards of material shall be excavated on areas with existing slopes greater than 15% in order to prepare the trenches for the proposed utility lines.

A site plan is included with this application showing all proposed improvements to be constructed under this special use permit.

#### PROJECT PHASING

Depending on several factors including available funding of the proposed improvements, the weather/constructable windows of time of the coming years, etc. – there is no set project phasing currently. However, given the scope of the proposed improvements, a time frame longer than two years is expected to be required.

Project Narrative September 2023

Sky Tavern - Special Use Permit

Washoe County, Nevada

#### **WATER RIGHTS**

A permit to appropriate water was granted for this project by The State of Nevada. A maximum amount of 48.3 acre-feet per year may be appropriated for recreational use per permit no. 91279; a copy of which is included in this application.

#### WETLANDS DELINEATION

Portions of this property include areas defined as wetlands that cannot be constructed on per the United States Army Corps of Engineers (USACE). A wetlands delineation report was prepared by Robison Wildlife Consulting, LLC, and the results of their findings are shown on the site plan. No improvements of any kind are proposed in any wetland areas.

#### TRAFFIC IMPACT

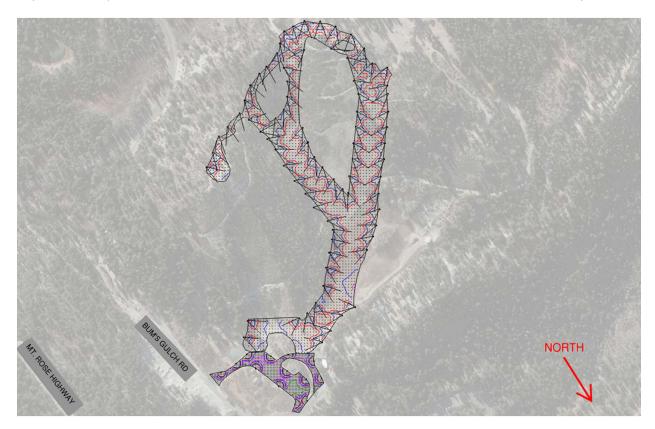
A traffic impact study was prepared for this project by Headway Transportation LLC and a copy is included with this application. Their study found that this expansion is anticipated to generate approximately 91 AM peak hour, and 43 Noon peak hour additional trips to the external roadway network,

#### **GEOTECHNICAL INVESTIGATION**

A geotechnical investigation was prepared for this project by Black Eagle Consulting, Inc. Their study found that the site is geotechnically suitable for the proposed improvements, and a copy of the report is included with this application.

#### SITE LIGHTING

A preliminary lighting design was prepared by Wisconsin Lighting Lab. Site lighting is proposed at the parking lot and all areas that will be served by the proposed snowmaking. Per the photometric below, the proposed lighting is not anticipated to impact neighboring properties or the adjacent streets, Mt. Rose Highway and Bum's Gulch Rd. Below is a preliminary photometric diagram excerpted from the preliminary design report by Wisconsin Lighting Lab.



The preliminary lighting design is included which details what specific lights are proposed.

#### LANDSCAPING AND REVEGETATION

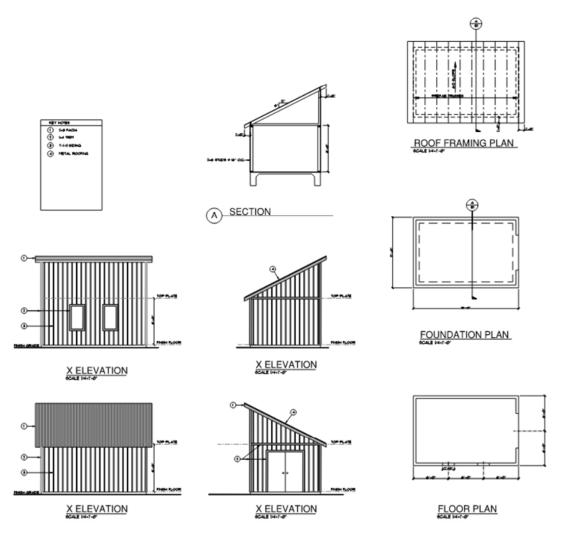
Sky Tavern has accumulated years of experience in promoting regeneration by native species and will treat all sloped areas with appropriate temporary erosion control, runoff management, and other Best Management Practices for all construction involving slopes or fresh disturbance. Revegetation with an aggressive for and native grass seed mix, followed by management of shrub and natural tree seedling regrowth, will achieve permanent stabilization consistent with the ski area operation.

#### **PARKING**

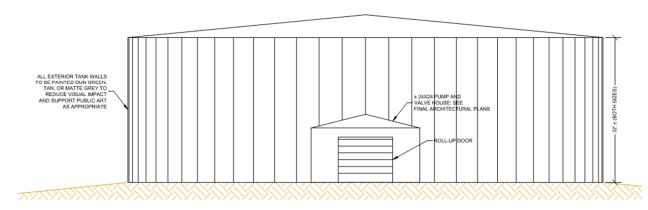
The existing parking area is proposed to be repaved and restriped. There is no specific parking requirement for a destination ski resort in Washoe County's development code, however Section 410.10.2 requires 1 parking space required per employee. A striping plan is included with this application showing 191 spaces.

#### PROPOSED STRUCTURES

The currently proposed structures include three pump house buildings and two water storage tanks. Architectural plans for the pump house buildings were prepared by Crom Engineering and for the water storage tanks by Robison Engineering. Below are the excerpted architectural drawings, and full sized plans are included with this application. Beyond what is shown on the plans, efforts shall be made to conceal the proposed structures so that they blend in with the surrounding aesthetic of the surrounding area, such as forest green paint, etc.



**Pump House Building Plans** 



Water Storage Tank Elevation View

#### **Washoe County Development Application**

Your entire application is a public record. If you have a concern about releasing personal information, please contact Planning and Building staff at 775.328.6100.

Project Information	s	taff Assigned Case No.:						
Project Name: Sky Tave	ern Junior S	Ski Program - Exp	ansion					
Project Expansion to e. Description: Program. Pleas	xisting destination se see project nar	n ski resort at Sky Tavern Junior Ski rrative for detailed description of project.						
Project Address: 21130 Mount Ros	se Hwy							
Project Area (acres or square fe	et): 143.07 acres							
Project Location (with point of re	ference to major cross	streets AND area locator):						
Property is southwest of the interse	ection of Mt. Rose Hwy	and Sky Tavern Rd; directly north	of Mt. Rose Ski Resort					
Assessor's Parcel No.(s):	Parcel Acreage:	Assessor's Parcel No.(s):	Parcel Acreage:					
048-050-03	143.070							
		s associated with this applicates to content is superceded by						
Applicant Inf	<b>ormation</b> (attach	additional sheets if necess	sary)					
Property Owner:		Professional Consultant:						
Name: City of Reno		Name: Robison Engineering Compa	iny, Inc					
Address: 1 E 1st St		Address: PO Box 1505						
Reno	Zip: 89501	Sparks	Zip: 89432					
Phone:	Fax:	Phone: (775) 852-2251	Fax: 852-9736					
Email:		Email: nathan@robisoneng.com						
Cell:	Other:	Cell: 775-240-7652	Other:					
Contact Person:		Contact Person: Nathan Earl Rob	ison, PE					
Applicant/Developer:		Other Persons to be Contact	ted:					
Name: Sky Tavern Junior Ski Area -	Applicant	Name:						
Address: 21130 Mt. Rose Hwy		Address:						
Reno	Zip: 89511		Zip:					
Phone: (775) 323-5125	Fax:	Phone:	Fax:					
Email: mike.oehlert@skytavern.com		Email:						
Cell:	Other:	Cell:	Other:					
Contact Person: Mike Oehlert		Contact Person:						
	For Office	Use Only						
Date Received:	Initial:	Planning Area:						
County Commission District:		Master Plan Designation(s):						
CAB(s):		Regulatory Zoning(s):						

#### Special Use Permit Application Supplemental Information

(All required information may be separately attached)

1. What is the project being requested?

Sky Tavern Junior Ski Program is proposing a new snowmaking system which shall include the construction of two new water wells, pump houses and water lines to supply two large water storage tanks which shall then supply the snowmaking hydrants.

2. Provide a site plan with all existing and proposed structures (e.g. new structures, roadway improvements, utilities, sanitation, water supply, drainage, parking, signs, etc.)

0'1				I
Site	pıan	IS	ınc	luded.

3. What is the intended phasing schedule for the construction and completion of the project?

Depending on several factors including available funding of the proposed improvements, the weather/constructable windows of time of the coming years, etc. – there is no set project phasing currently. However, given the scope of the proposed improvements, a time frame longer than two years is expected to be required.

4. What physical characteristics of your location and/or premises are especially suited to deal with the impacts and the intensity of your proposed use?

The property is a functional ski resort, and all proposed improvements are appropriate additions that serve the current use. The close proximity to existing electrical and sanitary utilities will allow for new services. The previously obtained water rights will allow for the snowmaking facilities and new water wells.

5. What are the anticipated beneficial aspects or affects your project will have on adjacent properties and the community?

The expansion will directly benefit the community as it will increase the facility's capacity, and extend the amount of time the ski program is operational throughout the year. More of the community will be able to take advantage of the recreational activity provided by Sky Tavern.

6. What are the anticipated negative impacts or affect your project will have on adjacent properties? How will you mitigate these impacts?

All of the new facilities are proposed onsite, and the construction phase will not negatively impact adjacent properties. Furthermore, given the seasonality of the ski resorts operations, much of construction can be completed without interrupting the typical schedule of the park.

7. Provide specific information on landscaping, parking, type of signs and lighting, and all other code requirements pertinent to the type of use being purposed. Show and indicate these requirements on submitted drawings with the application.

Please refer to the site plan and narrative document for specific information on landscaping, property of the site plan and narrative document for specific information on landscaping, property of the site plan and narrative document for specific information on landscaping, property of the site plan and narrative document for specific information on landscaping, property of the site plan and narrative document for specific information on landscaping, property of the site plan and narrative document for specific information on landscaping, property of the site plan and narrative document for specific information on landscaping, property of the site plan and narrative document for specific information on landscaping, property of the site plan and pl

8.	Are there any restrictive covenants, recorded conditions, or deed restrictions (CC&Rs) that apply to
	the area subject to the special use permit request? (If so, please attach a copy.)

□ Ves	■ No
	■ NO

#### 9. Utilities:

a. Sewer Service	EXISTING - SEPTIC
b. Electrical Service	EXISTING - NV ENERGY
c. Telephone Service	EXISTING - ATT & VARIOUS WIRELESS
d. LPG or Natural Gas Service	EXISTING - PROPANE
e. Solid Waste Disposal Service	EXISTING - WASTE MANAGEMENT CONTRACT
f. Cable Television Service	EXISTING - SATELLITE
g. Water Service	EXISTING - ON-SITE PUBLIC WATER SYSTEM

For most uses, Washoe County Code, Chapter 110, Article 422, Water and Sewer Resource Requirements, requires the dedication of water rights to Washoe County. Please indicate the type and quantity of water rights you have available should dedication be required.

h. Permit #	N/A	acre-feet per year	
i. Certificate#		acre-feet per year	
j. Surface Claim #		acre-feet per year	
k. Other #		acre-feet per year	

Title of those rights (as filed with the State Engineer in the Division of Water Resources of the Department of Conservation and Natural Resources).

The water storage tanks will have the largest visual impact and shall be painted to blend in with the surrounding area - see plans/narrative.

#### 10. Community Services (provided and nearest facility):

a. Fire Station	Truckee Meadows Fire & Rescue, Station 39
b. Health Care Facility	Renown Health; Incline Village Community Hospital; Tahoe Forest Hospital
c. Elementary School	Dodson Elementary School
d. Middle School	Marce Herz Middle School
e. High School	Galena High School
f. Parks	Galena High School
g. Library	South Valleys Library
h. Citifare Bus Stop	Herz Boulevard and Mt. Rose Highway

### Special Use Permit Application for Grading Supplemental Information

(All required information may be separately attached)

	avation of utility trenches, grading required for the construction of two large er storage tanks - see plans.
How r	many cubic yards of material are you proposing to excavate on site?
Αŗ	oprox. total cut volume = 6,600 CY
How r	many square feet of surface of the property are you disturbing?
Com	abined estimated disturbed area including paving = 6.1 acres (265,000sf)
	many cubic yards of material are you exporting or importing? If none, how are you managing to ce the work on-site?
repla	export or import is proposed. All material excavated for pipe trenches shall be aced/recompacted. The grading associated with the construction of the water age tanks has a balanced cut/fill.
	ossible to develop your property without surpassing the grading thresholds requiring a Special Permit? (Explain fully your answer.)
No -	the excavation volume for the utility trenches exceeds standards.
	any portion of the grading shown on the plan been done previously? (If yes, explain the instances, the year the work was done, and who completed the work.)
Non	e of the proposed grading has been performed.
	you shown all areas on your site plan that are proposed to be disturbed by grading? (If no in your answer.)
Yes	· · · · · · · · · · · · · · · · · · ·

8.	Can the disturbed area be seen from off-site? If yes, from which directions and which properties or roadways?
	The disturbed areas will be visible from the east, from Mt. Rose Highway
9.	Could neighboring properties also be served by the proposed access/grading requested (i.e. if you are creating a driveway, would it be used for access to additional neighboring properties)?
	These improvements do not directly serve neighboring properties, however the fire access roads and elevated water storage are of potential regional value.
10.	What is the slope (horizontal/vertical) of the cut and fill areas proposed to be? What methods will be used to prevent erosion until the revegetation is established?
	All proposed graded slopes are max 3:1.
11.	Are you planning any berms?
	Yes No X If yes, how tall is the berm at its highest?
12.	If your property slopes and you are leveling a pad for a building, are retaining walls going to be required? If so, how high will the walls be and what is their construction (i.e. rockery, concrete, timber, manufactured block)?
	No walls are proposed.
13.	What are you proposing for visual mitigation of the work?
	The water storage tanks will have the largest visual impact and shall be painted to blend in with the surrounding area - see plans/narrative.
14.	Will the grading proposed require removal of any trees? If so, what species, how many and of what size?
	The construction of the water storage tanks is expected to remove approximately 10 trees consisting of Jeffrey Pines and White Firs, with trunk sizes ranging from 1" to 18" within the proposed earthwork areas.
15.	What type of revegetation seed mix are you planning to use and how many pounds per acre do you intend to broadcast? Will you use mulch and, if so, what type?

Dryland grass and shrubs suitable for high elevation slope stabilization and low fuel content.

16. How are you providing temporary irrigation to the disturbed area?

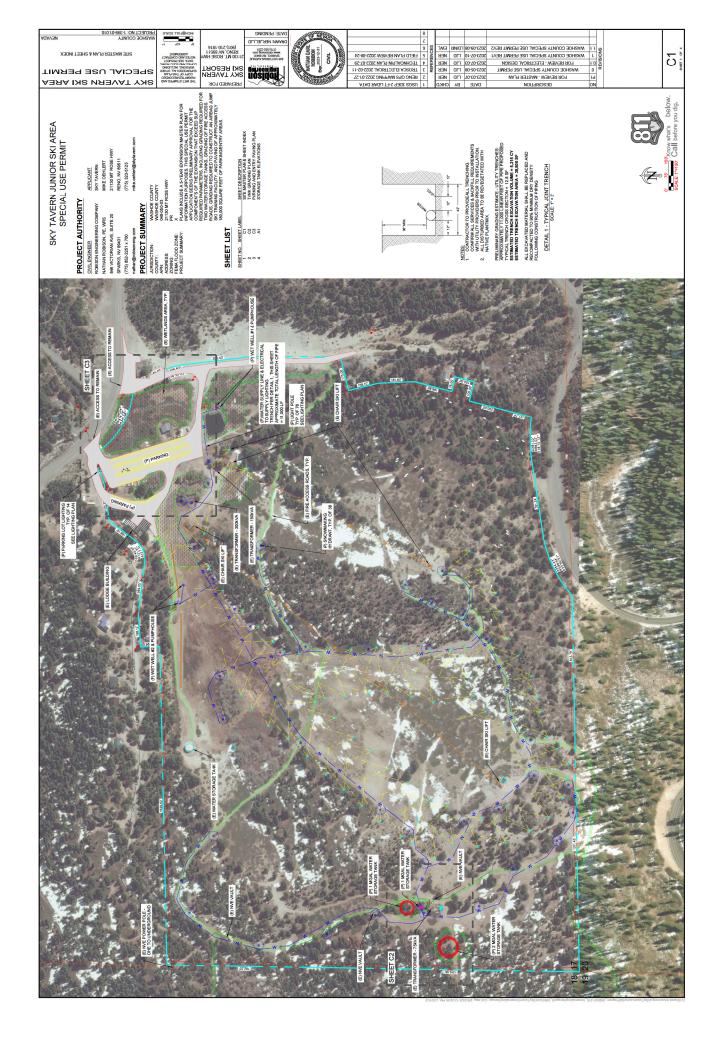
Temporary irrigation shall be provided if required for the disturbed earthwork associated with the construction of the water storage tanks.

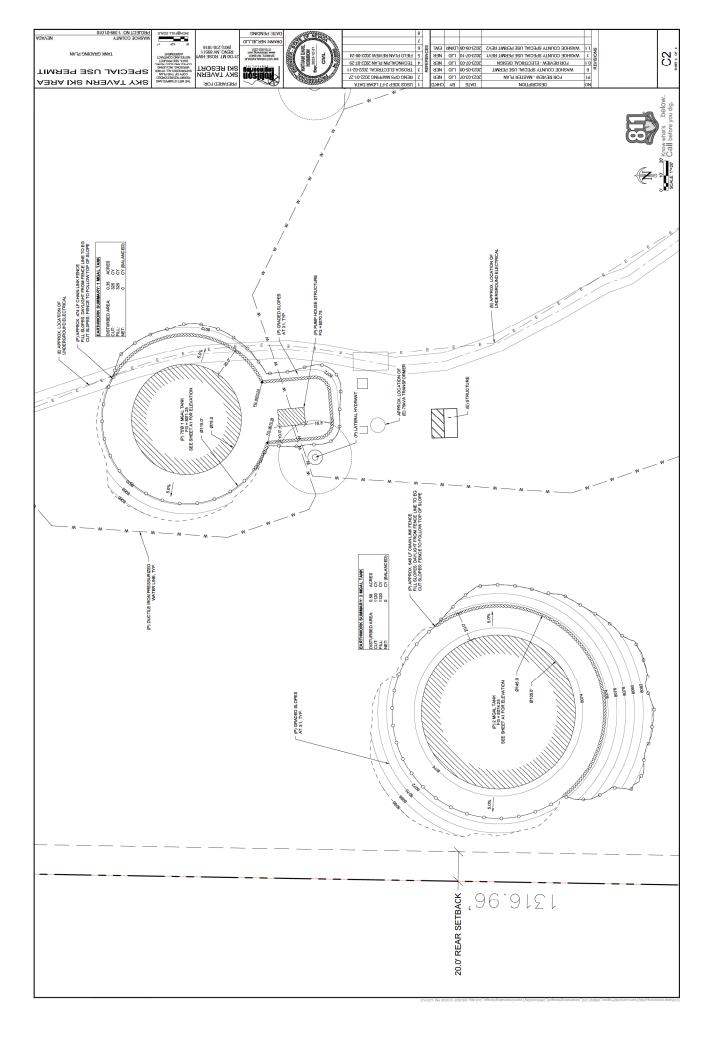
17. Have you reviewed the revegetation plan with the Washoe Storey Conservation District? If yes, have you incorporated their suggestions?

The plan will be submitted to the Washoe Storey Conservation District prior to grading permit applications.

18. Are there any restrictive covenants, recorded conditions, or deed restrictions (CC&Rs) that may prohibit the requested grading?

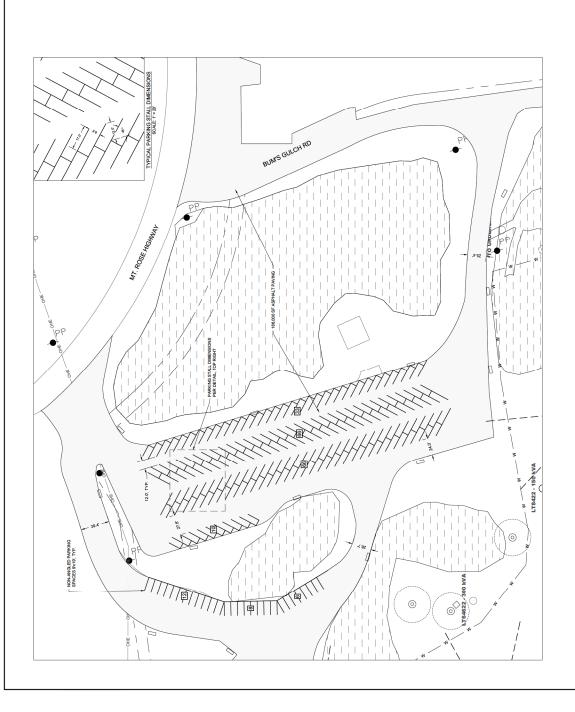
	NI - V	16 and also a standard standard
Yes	NoX	l If yes, please attach a copy.

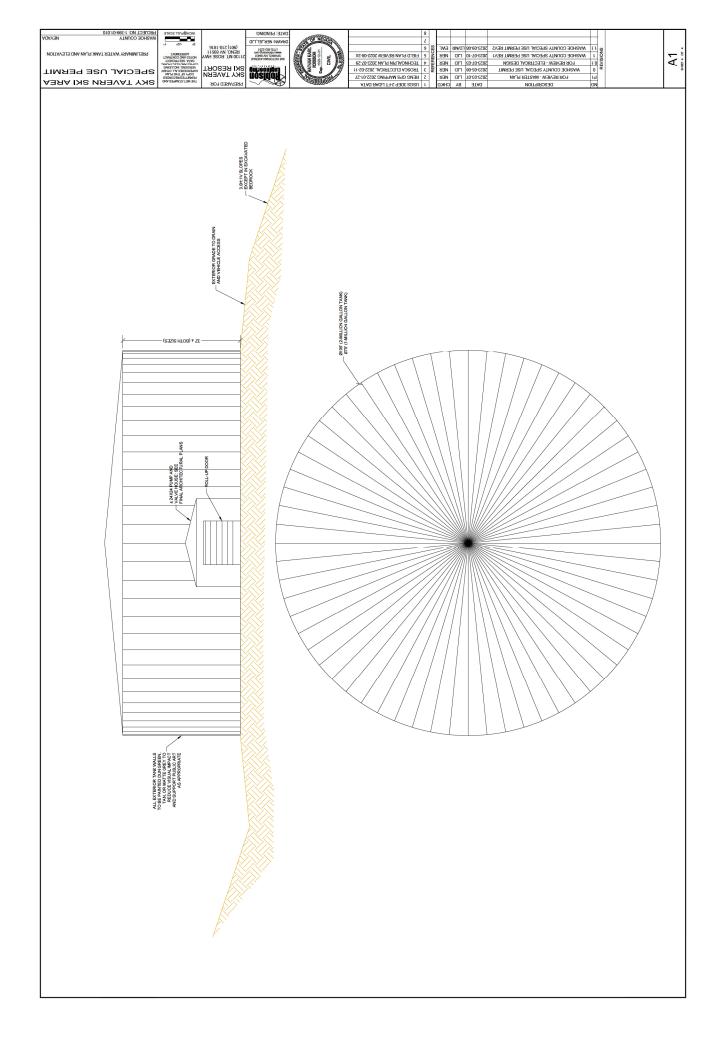




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PARKING AND ENTRY PAVING PLAN		21130 MT. ROSE HWY	SPARSCS, NV 89431 SPARSCS, NV 89431	CIVIL CIVIL	HELD PLAN REVIEW 2022-08-24	9 👸	NEB	σm	2023-07-10	WASHOE COUNTY SPECIAL USE PERMIT REVI	ı		3
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Mr. Nathan Robison, P.E. Robison Engineering Co. 846 Victorian Avenue #20 Sparks, Nevada 89431 Project No.: 1674-17-1 August 24, 2023

RE: Geotechnical Summary
Sky Tavern Ski Area; APN 048-050-03
Washoe County, Nevada

Dear Mr. Robison:

Black Eagle Consulting, Inc. (BEC) is pleased to present this geotechnical summary of the proposed Sky Tavern Ski Resort improvements project to be constructed within the overall ski resort premises located on Mt. Rose Highway in Washoe County, Nevada. The projects are in initial planning and development, and an application for a special use permit is planned to be submitted to Washoe County. The projects will ultimately involve the design and construction of numerous proposed improvements and modifications to the existing ski area which include: new asphalt and concrete paving for parking areas adjacent to Mt. Rose highway, a new ski lift, an expansion to the existing lodge building, construction of a new Sprung® Structure building adjacent to the parking area, underground utilities for snowmaking, site lighting, a 1-million-gallon and a 2-million-gallon water storage tank, grading of new roads and pathways, a maintenance building, a ski jump and a skateboard park, and various ancillary buildings throughout the developed portions of the property. This geotechnical summary is to be used for planning and special use submittal purposes and will need to be supplemented with field exploration, laboratory testing and geotechnical analyses for the final design and construction of the specific improvements. It is our understanding that the improvements will be completed in phases.

Black Eagle Consulting, Inc. previously completed a geotechnical investigation for the existing water tank at Sky Tavern Ski Resort. Field logs and laboratory testing from the previous investigation have been used to supplement our geotechnical summary.

#### **Project Description**

As noted above, the project will involve the design and construction of numerous proposed improvements throughout the overall Sky Tavern Ski Resort site. The proposed 1- and 2-million-gallon water tanks will be of bolted steel construction which will be founded on Portland cement concrete (PCC) shallow ring foundations, and the bases will be placed on compacted aggregate base. Cuts and fills of up to 9 feet will be required to create level pads for the tanks. The proposed buildings and ancillary structures will be supported on shallow PCC foundations with PCC slab-on-grade floors. Site improvements include installation of a snowmaking system with buried water lines and aboveground snowmaking towers, a new ski lift, a new magic carpet lift, and construction of paved and unpaved access roads and pathways. Grading for these improvements will vary but is generally expected to involve less than 10 feet of cuts and fills. It is unclear if retaining walls will be necessary.

Mr. Nathan Robison, P.E. Robison Engineering Co. August 24, 2023

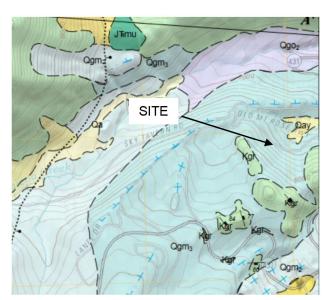
#### **Site Conditions**

The overall Sky Tavern Ski Resort site is located in the Carson Range between Reno and Lake Tahoe and consists of approximately 143 acres of mountainous terrain on Mount Rose in Washoe County, Nevada. The site is located in Sections 17 and 18, Township 17 North, Range 19 East, Mt. Diablo Meridian. The ski area is located to the west and north of Mt. Rose Highway (State Route 431).

The overall site includes steep terrain on Mount Rose sloping down to the east, towards Galena Creek, and to the south, towards Brown's Creek. Overall vertical relief is approximately 650 feet, but local relief within specific project areas is limited to less than 40 feet. The land is moderately to heavily vegetated by pine forest with native underbrush. Multiple stream drainages transect the project area, generally sloping towards the east, which are vegetated by aspen groves and alpine plants. The base area is located near the northeastern corner of the property and includes asphalt-paved parking and drives as well as multiple buildings such as the lodge building and numerous ancillary structures for maintenance and storage. Public utilities are present near the base area which include municipal water, electrical and sewer.

#### **Geologic and General Soil Conditions**

The site lies on the eastern slopes of Mount Rose in an area mapped by the Nevada Bureau of Mines and Geology (NBMG) as Glacial Deposits (Late Pleistocene), Undivided Granitic Pluton(s) (Cretaceous), Young Alluvial Deposits (Late Pleistocene to Holocene), Glacial Outwash Deposits (Late Pleistocene) (Hinz et al., 2014). In general, the vast majority of the site is underlain by granitic bedrock; however, the areas where the majority of development are planned are surfaced by glacial deposits, with granitic outcroppings generally exposed at and near peaks within the site. The glacial deposits are described as Unsorted or poorly sorted sand, gravel and boulders (till). Granitic rocks typically moderately to highly weathered. The granitic plutons are described as granite, granodiorite, quartz



Hinz et al., 2014

monzonite, monzodiorite, and diorite. Locally contains abundant mafic enclaves (up to 50% by volume).

Based on previous exploration in the area, the site materials are generally non-plastic silty sand with gravel soils in a cobble-boulder matrix. The granitic bedrock typically has a weathering rind several feet thick of grus, making it excavatable through intermediate depths (5 to 10 feet). Hard bedrock materials can be present locally.

Mr. Nathan Robison, P.E. Robison Engineering Co. August 24, 2023

While groundwater lies at considerable depths, seasonal snowmelt contributing to perched groundwater is common during spring and early summer and is possible through fractured rock at any time of year.

#### **Geologic Hazards**

The Sky Tavern Ski Resort lies within an area with a high potential for strong earthquake shaking. Seismic design criteria for proposed structures will be addressed in the future geotechnical investigation(s) for specific projects that will be performed by BEC.

No earthquake hazards map is available for the project location. The geologic map (Hinz et al., 2014) and the United States Geological Survey (USGS) U.S. Quaternary faults web mapping tool (USGS, 2023) identify the nearest fault lying approximately 1,200 feet to the east of the parking area and lodge. This fault, the Little Valley Fault, is mapped as being Holocene in age. Based on the available mapping, no additional fault investigation or mitigation in the form of building/structure setbacks is necessary for the structures proposed in the improvements.

The area is mapped as Zone X, or areas determined to be outside of the 500-year floodplain (Federal Emergency Management Area [FEMA], 2009a and b). However, multiple drainages exist within the site and at times may carry large volumes of water and debris flows which are able to breach the banks and any existing stormwater infrastructure.

The site should exhibit a moderate potential for dust generation during dry months. Rock slide/debris flow hazards are possible based on local terrain and can be mitigated with proper engineering. No other geologic hazards were identified.

#### **Discussion and Conclusions**

The site is geotechnically suitable for the proposed improvement projects. Soils within the site are expected to be granular sand and gravel deposits with considerable cobbles and boulders. Isolated areas (water tanks, new lift towers and water line) should expect to encounter granitic bedrock with varying degrees of weathering/hardness. The granular soils and granitic bedrock will provide excellent foundation support for the proposed improvements when properly prepared. When excavated, the native granular materials can be reused as structural fill, although removal of oversized particles may result in significant quantity shrinkage. The presence of bedrock, cobbles and boulders should be anticipated and will result in excavation difficulty. The geotechnical constraints associated with oversized cobbles and boulders or bedrock will be addressed in BEC's geotechnical investigation report(s).

#### Closing

This report has been prepared in general accordance with accepted geotechnical practices to provide an overall geotechnical summary and expected constraints for planning of the proposed improvements. As noted earlier, individual, project-specific geotechnical investigations will be completed with detailed geotechnical recommendations for the design and construction of the proposed improvements.

Mr. Nathan Robison, P.E. Robison Engineering Co. August 24, 2023

We appreciate having the opportunity to work with you on this project. If you have any questions regarding our findings, please contact us.

Sincerely,

Black Eagle Consulting, Inc.

CHARLES
REMINGTON
WALKER
EXP: 06-30-24
CIVIL
Vo. 27125

C. Remington Walker, P.E. Project Engineer

CRW:JP:cjr

Copies to:

Addressee (PDF via Email)

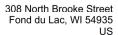
#### References

Federal Emergency Management Agency (FEMA), 2009a (March 16, 2009), Flood Insurance Rate Map 32031C3325G Washoe County, Nevada.

FEMA, 2009b (March 16, 2009), Flood Insurance Rate Map 32031C3350G Washoe County, Nevada.

Hinz, N.H., Ramelli, A.R., and Faulds, J.E., 2014, *Preliminary Geologic Map of the Mt. Rose Quadrangle, Washoe County, Nevada,* Nevada Bureau of Mines and Geology, Open File Report 14-7

United States Geological Survey (USGS), 2023, Online Quaternary Fault and Fold Database of United States, Google Earth Files at https://earthquake.usgs.gov/hazards/qfaults/ accessed August 2023.





#### **QUOTATION**

Quotation Details

Job Name:

Sky Tavern Ski Area-

Date Created: 6/26/23

Quote #: 972928

Exp. Date: 7/14/23

**Contact Information** 

 Prepared By:
 Tyson Karcheski
 Contact Name:
 Mike Oehlert

 Phone #:
 (866) 308-9455
 Phone #:
 775-848-3992

E-Mail: customer.quotes@willbrands.com Email: mike.oehlert@skytavern.com

Fax: 920-921-0781 Fax:

Address Information

Bill To Name: Sky Tavern Ski Area (1438029) Ship To Name: Sky Tavern Ski Resort

 
 Bill To:
 21130 MT Rose Hwy Reno, NV 89511
 Ship To:
 21130 Sky Tavern Rd Reno, NV 89511

#### **Standard Products**

Qty	Product	Sale Price	Ext. Price
77	[PART#] VS-RTSA-35-7324-11-AB-GV [DESCRIPTION] 35' Tall x 7.3" Base x 2.4" Top x 11ga Thick, Round Tapered Steel, Anchor Base Light Pole, Full Base Cover, Wiring Hand Hole & Cover, Standard Fixture Mounting & Durable Hot-Dip Galvanized Finish, USA Engineered & Manufactured (Includes 1" Anchor Bolts) [COMMENTS] SKI RUN POLES	\$2,199.00	\$169,323.00
77	[PART#] VS-S-BLH-R24-2-180-GV [DESCRIPTION] 2 @ 180 deg. Fixture Mount, Steel Bullhorn Bracket, Pole Top Mount, Hot-Dip Galvanized Finish, USA Engineered & Manufactured [COMMENTS] SKI RUN BULLHORNS	\$299.00	\$23,023.00
77	[PART#] VS-P2 [DESCRIPTION] Light Fixture Mounting, 2.38" OD x 4" Long, Tenon Top [COMMENTS] ALL POLES	\$0.00	\$0.00
77	[PART#] VS-PRE100 [DESCRIPTION] Pre-Shipped Anchor Bolt Kit, 1" x 36" x 4" [COMMENTS] SKI RUN PRE-SHIPPED ANCHORS	\$99.00	\$7,623.00
154	[PART#] NF-SHM-250-40-MV-45-DB-SF-WHP7NP [DESCRIPTION] 250W, NAFCO® Medium SHX Shoebox LED Light Fixture, 40000 Lumens, 4000K, 120-277V Input VAC, 45° Medium Spot Distribution [COMMENTS] [DARK BRONZE][SLIPFITTER][WHP7NP][GLARE CUPS]	\$775.00	\$119,350.00
154	[PART#] OA-SF [DESCRIPTION] Adjustable Slipfitter Mount, Fits 2.38" OD Tenon & Arm Mounts, Dark Bronze Powder Coat Standard [COMMENTS] SKI RUN SLIP FITTERS FOR FIXTURES	\$40.00	\$6,160.00
154	[PART#] OA-WHP7NP [DESCRIPTION] 6' Cord w/o Plug, Stripped Pigtail [COMMENTS] WHIP CHORD FOR SKI RUN FIXTURES	\$0.00	\$0.00
13	[PART#] VS-RSSA-18-4040-11-AB-GV-P2 [DESCRIPTION] 18' Tall x 4.0" OD x 11ga Thick, Round Straight Steel, Anchor Base Light Pole, Full Base Cover, Wiring Hand Hole & Cover, Standard Fixture Mounting & Durable Hot-Dip Galvanized Finish, USA Engineered & Manufactured (Includes 3/4" Anchor Bolts) [COMMENTS] PARKING LOT POLES	\$1,059.00	\$13,767.00
13	[PART#] VS-S-BLH-R24-2-180-GV [DESCRIPTION] 2 @ 180 deg. Fixture Mount, Steel Bullhorn Bracket, Pole Top Mount, Hot-Dip Galvanized Finish, USA Engineered & Manufactured [COMMENTS] PARKING LOT BULLHORNS	\$299.00	\$3,887.00
13	[PART#] VS-PRE075 [DESCRIPTION] Pre-Shipped Anchor Bolt Kit, 0.75" x 17" x 3" [COMMENTS] PARKING LOT PRE-SHIPPED ANCHORS	\$35.00	\$455.00
26	[PART#] NF-SHS-120-40-MV-4-DB-SF [DESCRIPTION] 120W, NAFCO® Small SHX Shoebox LED Light Fixture, 20250 Lumens, 4000K, 120-277V Input VAC, Type 4 Distribution	\$499.00	\$12,974.00

	[COMMENTS] PARKING LOT FIXTURES [DARK BRONZE][SLIPFITTER		
26	[PART#] OA-WHP3NP [DESCRIPTION] 2' Cord w/o Plug, Stripped Pigtail [COMMENTS] WHIP CHORD FOR PARKING LOT FIXTURES	\$0.00	\$0.00
26	[PART#] OA-SF [DESCRIPTION] Adjustable Slipfitter Mount, Fits 2.38" OD Tenon & Arm Mounts, Dark Bronze Powder Coat Standard [COMMENTS] PARKING LOT SLIP FITTERS FOR FIXTURES	\$40.00	\$1,040.00

- Pricing includes delivery within the contiguous USA unless otherwise noted and is based on an order release within 30 days.
- Preshipped anchor bolts at CUSTOMER'S expense.
- Sales tax calculation (if applicable) will be finalized at order entry (OE).
- If order quantity & quote quantity are different, pricing is subject to change.
- Quote is subject to Wisconsin Lighting Lab's standard terms and conditions. See website for complete details

Grand Total: \$357,602.00

Estimated Lead Time: 10 to 12

Weeks

#### Important Notes

- \*\*\*MADE IN THE USA\*\*\*
- \*\*\*GALVANIZED POLES AND BULLHORNS\*\*\*
- \*\*\*PRE-SHIPPED ANCHORS INCLUDED\*\*\*
- \*\*\*QUOTING 120-277 VOLTS (CUSTOMER TO CONFIRM\*\*\*
- \*\*\*CUSTOM SHORT GLARE CUP INCLUDED WITH FIXTURES\*\*\*
- \*\*\*SPEC SHEETS AND PHOTOMETRICS ATTACHED\*\*\*
- \*\*\*PRICE SHOWN IS THE DELIVERED COST MINUS TAX\*\*\*
- Light pole base: Assumes new install
- Loading Assumption: light poles rated for 7.1 EPA, 177 LBS at 100 MPH.
- Loading Assumption: mounting quantity [2, light fixtures]; [1, bracket] per pole.
- Lightning strikes and voltage surges can cause LED fixture damage; additional surge protection available upon request.
- Non-Standard specifications may add to the production lead time.
- Customer responsible for unloading at time of delivery; line items may ship at different times during production cycle.
- Commercial shipping location with receiving dock can be provided to void limited access delivery charges.
- Quoted Lead time is an estimate and is subject to change as business conditions change.

IMPORTANT: Do you require vibration dampeners? We recommend vibration dampeners be used when (1) light poles are being installed on a parking ramp, deck, bridge, pier, airport, train or subway hub/terminal or known problem area (2) a load of 0.75 EPA or smaller is going on the light poles and (3) light poles are being used as camera supports and/or will have non-standard appendages attached to them.

IMPORTANT: Wisconsin Lighting Lab and its vendors are not responsible for the structural adequacy of new and/or existing light pole footing designs and anchor bolts. Estimated loading capacity values and wind zone ratings are based on standard commercial design and engineering criteria, and they do not account for additional loadings from objects such as (but not limited to) signs, banners, cameras, solar panels and flags. Our light pole warranty does not cover vibration induced fatigue failure.

IMPORTANT: Wisconsin Lighting Lab and its vendors consider these quoted products as produced and supplied according to the customer's dimensional, material and/or electrical specifications. To ensure proper selection of the light pole, luminaire, accessories and/or foundation, we recommend the customer consult a qualified local engineer to analyze the loading, design and project criteria for the specific application.



P: 866-308-9455 customer.quotes@willbrands.com

308 North Brooke Street Fond du Lac, WI 54935 US





# Sky Tavern Ski Area

06/19/2023

Job Number: 972928

Prepared by: CO

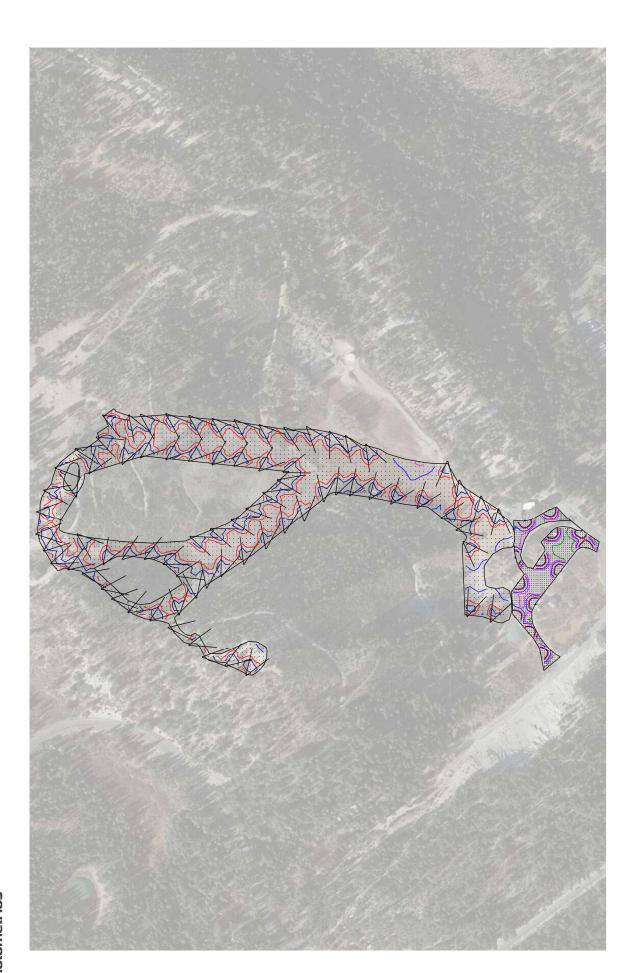
NOTES

Powered by Wisconsin Lighting Lab, Inc.

\*Luminaire testing data is based on Illuminating Engineering Society (IES) standards under simulated and laboratory conditions. This design is based on information supplied by others, and richdokal lafeld measurements may vay if from computer-standards dealcalierions due to variables life (but not intend to) variation in electrical voltage, environmental conditions and other variable field characteristics. Plycoal field foot cande measurements may vary +/- 10%. For sports lighting field measurements should be taken in accordance with IESNA RP-6-15. Conformance to facility and local codes is the responsibility of the owner and their representatives. This layout may not meet CA Title 24 and/or other focal energy codes. If specific compliance is required, those details must be provided to your facility despit representative.

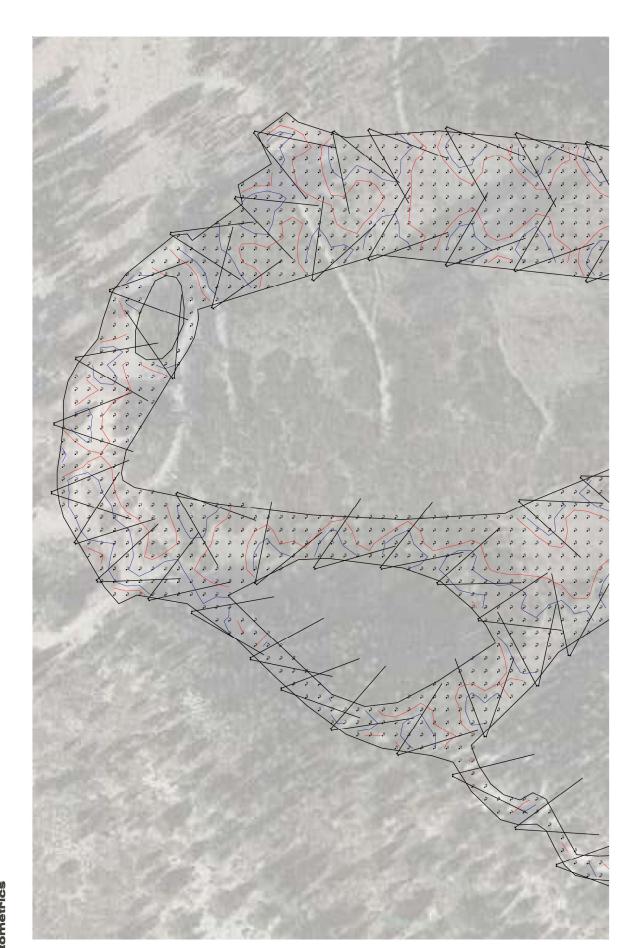
\*\*Satisfactory performance and safe use of LED sports lighting fixtures is dependent upon light poles, brackets, anchorage and other structural components being of adequate design and condition. The total combined Effective Projected Area (EPA) and weight of all fixtures, prackets and attachments mounting to a light pole cannot exceed the EPA and weight atting for a specified pole. For sports lighting retroffit applications, it is the customer's responsibility to have a qualified inspector and/or engineer confirm the structural adequacy of the existing light poles assemblies. We are happy to quote new light poles and brackets if you have concerns about your existing materials, bonded



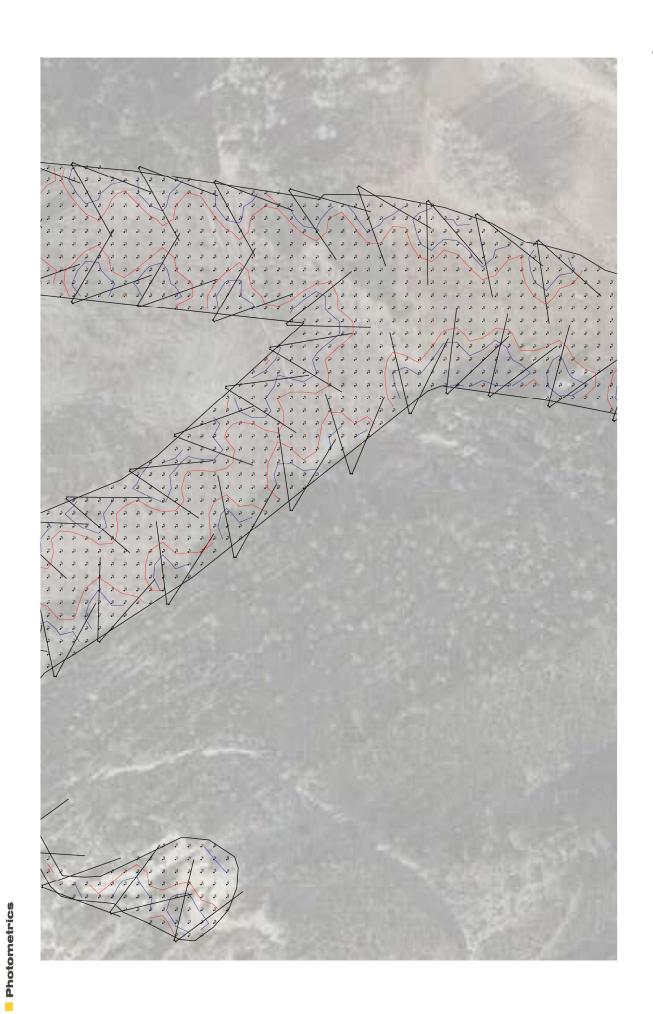




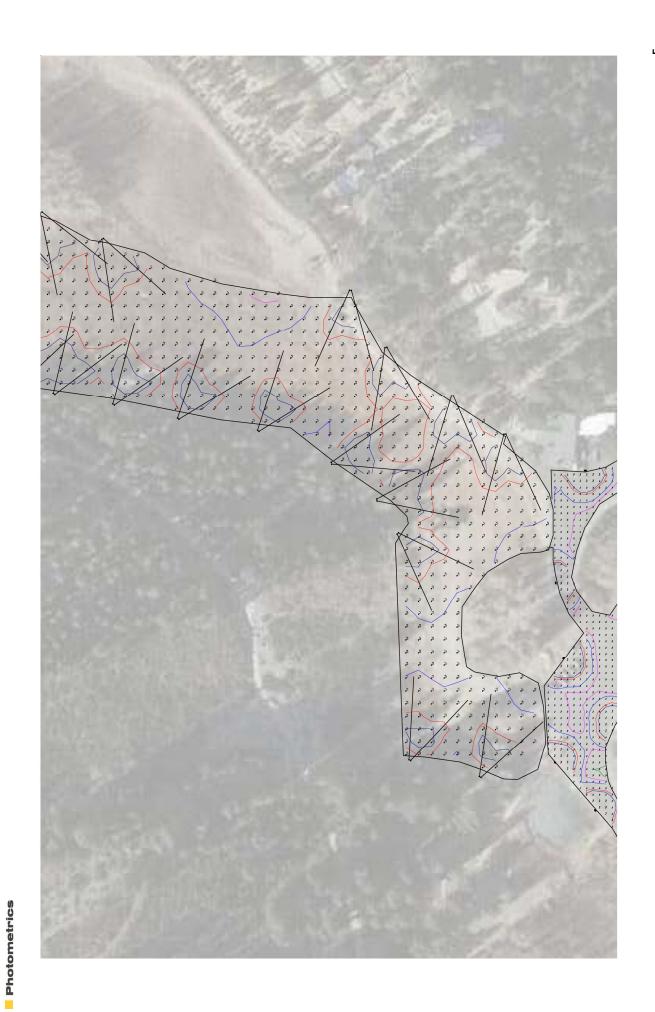
# Photometrics



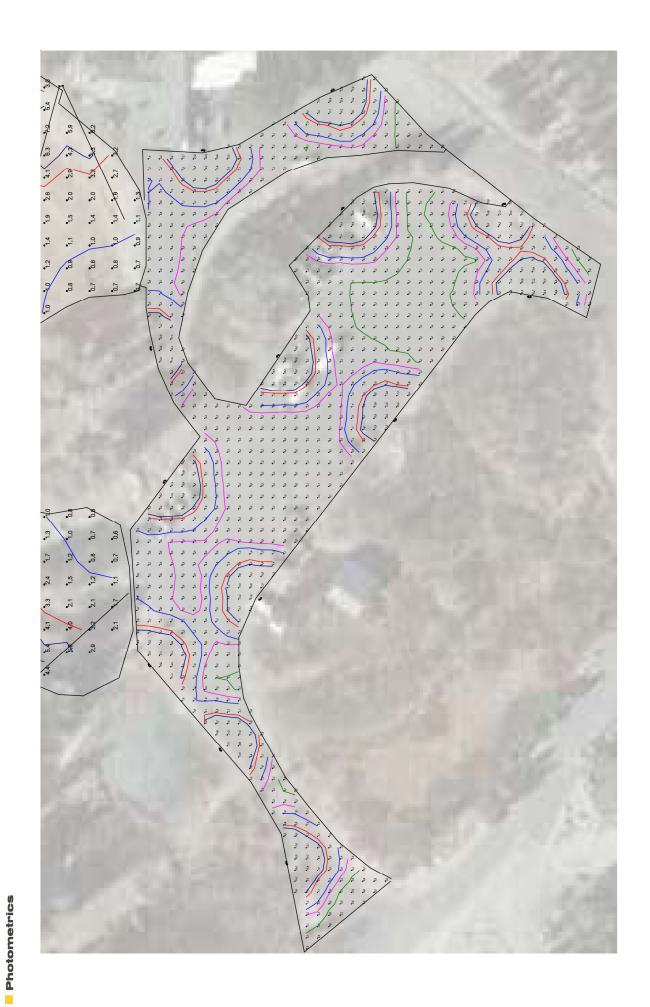














308 N. Brooke St. | Fond du Lac, WI 54935 quotes@willbrands.com | www.willbrands.com ©2022



## Calculations

### Value Illuminance (Fc) **Soline Legend** 0.1 0.5 Color 0.950 0.950 片 NF-SHS-120-40-xx-4 Labe 7 Luminaire Schedule 흏 Symbol

# Calculation Summary

3.5

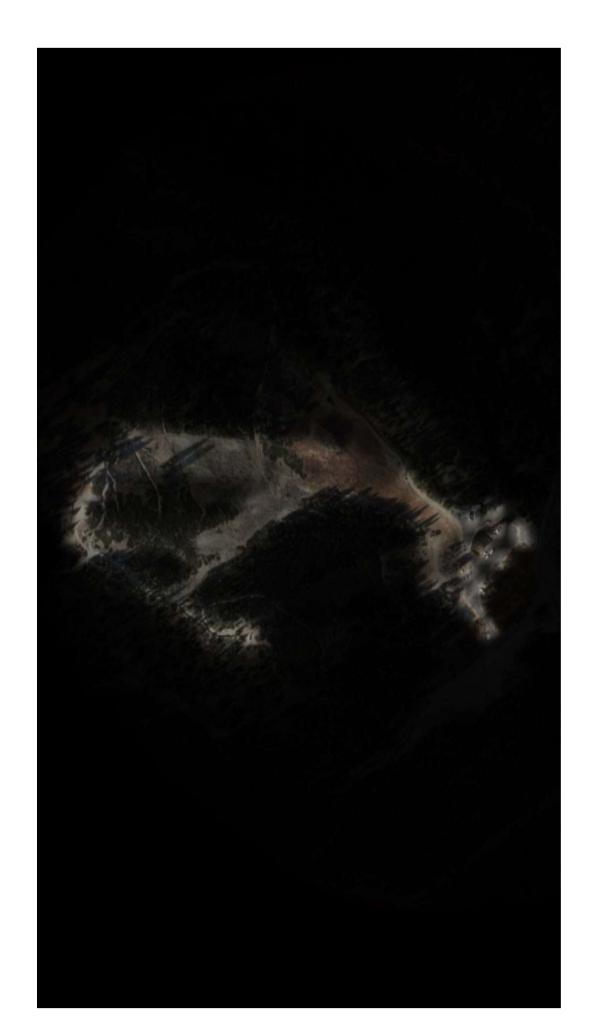
2

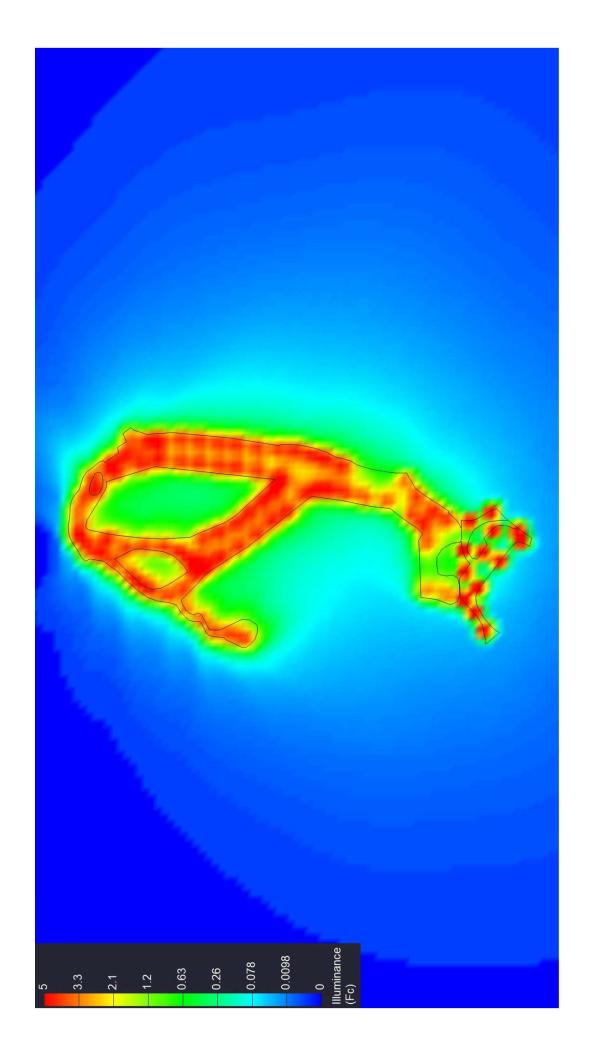
Label	CalcType	Units	Avg	Max Min	Min	Avg/Min Max/Min	Max/Min
Parking Lot	Illuminance	S.	3.32	42.7	0.0	42.7 0.0 N.A. N.A.	N.A.
Phase 1 Lighting	Illuminance	S	3.74	9.1	0.5	3.74 9.1 0.5 7.48 18.20	18.20

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Renderings









#### NAFCO® RTSA ROUND TAPERED STEEL ANCHOR BASE LIGHT POLES









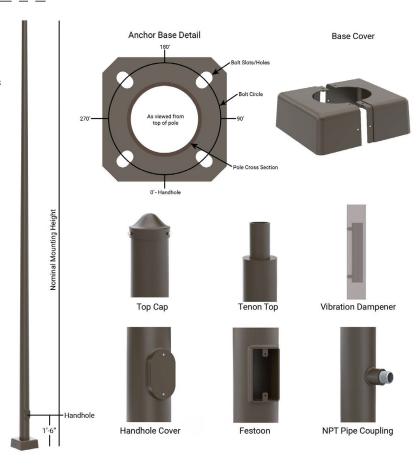




Proudly engineered and manufactured in the American Midwest - our NAFCO® family of professional-grade light pole products combines 50+ years of manufacturing expertise and top-notch Midwestern workmanship. Like all WiLL products, NAFCO® poles come supported by our unmatched design, engineering, and project support capabilities.

#### Specifications

- · Pole Shaft The pole shaft is a one-piece assembly conforming to ASTM A595 Grade A or A572 Grade 55 with a constant linear taper of 0.14 in/ft. Poles greater than 50' are provided as a two-piece field assembled unit.
- Pole Top A removable top cap is provided for poles receiving drilling patterns for side-mount luminaire arm assemblies. Other pole top options include Tenon Top, Top Cap Only or Open Top which is typical when the pole top diameter matches the necessary slip-fit dimensions.
- Hand Hole A reinforced hand hole with grounding provision is provided at 1'-6" from the base end of the pole assembly. Each hand hole includes a cover and cover attachment hardware. Poles with a 5.90" base diameter are supplied with a 3" X 5" rectangular hand hole. All other pole assemblies are provided with a 4" X 6.5" oval hand hole (dimensions are
- Base Cover A two-piece full base cover fabricated from ABS plastic is provided with each pole assembly. Additional base cover options, including a cast aluminum and fabricated steel cover, are available upon request. A two-piece steel full base cover is required for some applications depending on the finish requirement and/or pole base square. Factory reserves the right to provide a two-piece steel full base cover on some applications depending upon the finish requirement and/ or pole base diameter.
- Anchor Bolts Anchor bolts conform to ASTM F1554 Grade 55 and are provided with two hex nuts and two flat washers. Bolts have an "L" bend on one end and are galvanized a minimum of 12" on the threaded end.
- Nut Covers Nut covers can be substituted on most models.
- Hardware All structural fasteners are galvanized high strength carbon steel. All non-structural fasteners are galvanized or zinc-plated carbon steel or stainless steel.
- Finish Standard finishes are either Galvanized or Finish Painted. Additional finish options including Finish Paint over Galvanizing are available upon request.
- Design Criteria Standard EPA (Effective Projected Area) and weight values are based on Standard Commercial Criteria (with 1.3 gust factor) for top mounted fixtures only. Consult the factory on loading criteria for side mounted luminaires and/or brackets. Satisfactory performance of light poles is dependent upon the pole being properly attached to a supporting foundation of adequate design.





#### EPA Loading Guide (Commercial Design Criteria)

	80mph w	/ 1.3 Gust	90mph w	/ 1.3 Gust	100mph v	v/ 1.3 Gust	110mph v	w/ 1.3 Gust	120mph v	w/ 1.3 Gust	130mph v	w/ 1.3 Gust	140mph v	v/ 1.3 Gust	150mph v	w/ 1.3 Gust
Base Model	Max EPA (sq ft)	Max Weight (lb)														
VS-RTSA-20-5931-11-AB-FP	19.3	482	15.1	377	12.2	305	10.5	262	8.8	220	7.4	185	6.3	157	5.5	138
VS-RTSA-20-6537-11-AB-FP	24.2	605	19.3	482	15.6	390	13.5	335	11.2	280	9.5	238	8.1	203	6.9	173
VS-RTSA-25-5924-11-AB-FP	12.5	312	9.9	247	8	200	6.9	173	5.7	143	4.8	119	4.1	103	3.5	88
VS-RTSA-25-7035-11-AB-FP	20.3	507	16.2	405	13.1	327	11.2	280	9.4	235	7.8	194	6.7	168	5.7	143
VS-RTSA-25-7035-07-AB-FP	30.5	760	24	625	19.8	495	16.5	413	13.9	348	11.7	293	10.0	250	8.5	213
VS-RTSA-30-6624-11-AB-FP	11.7	292	9.3	232	7.5	187	6.5	163	5.4	135	4.6	115	3.9	98	3.3	83
VS-RTSA-30-8038-11-AB-FP	18.9	473	14.9	373	12	300	10.0	250	8.4	210	6.8	168	5.7	143	4.7	118
VS-RTSA-30-8038-07-AB-FP	33.5	838	27	675	22	550	18.4	460	15.3	381	13.0	325	11.0	275	9.3	233
VS-RTSA-35-7324-11-AB-FP	11.2	280	8.9	222	7.1	177	6.3	156	5.3	131	4.4	110	3.8	95	3.1	78
VS-RTSA-35-8536-11-AB-FP	18.9	472	15.1	377	12.2	305	10.5	263	8.7	218	7.2	180	5.9	148	4.8	120
VS-RTSA-35-9546-11-AB-FP	23.2	580	18.2	455	14.5	363	12.0	300	9.7	243	7.8	194	6.3	156	5.2	130
VS-RTSA-39-7824-11-AB-FP	10.7	267	8.5	212	6.6	165	5.7	143	4.5	113	3.7	93	2.9	73	2.3	58
VS-RTSA-39-9036-11-AB-FP	17.2	430	13.5	338	10.8	270	9.0	225	7.3	183	5.8	144	4.7	118	3.8	95
VS-RTSA-39-9036-07-AB-FP	28.5	715	23	575	19	475	15.8	394	13.2	330	10.8	269	9.0	225	7.7	193
VS-RTSA-45-1037-11-AB-FP	17.4	435	13.5	338	10.6	265	8.8	219	7.0	175	5.5	138	4.3	108	3.2	80
VS-RTSA-45-1037-07-AB-FP	28.5	715	23	575	19	475	16.0	400	13.3	333	11.0	275	9.0	225	7.4	185
VS-RTSA-45-1147-07-AB-FP	35.7	893	28	700	22.3	558	18.7	468	15.0	375	12.2	305	10.0	250	8.1	203
VS-RTSA-50-1030-11-AB-FP	13.2	330	10.6	265	8.3	208	6.9	173	5.4	135	4.1	103	3.1	78	2.2	55
VS-RTSA-50-1030-07-AB-FP	20.5	512	16.5	412	13.6	340	12.1	303	10.2	255	8.7	218	7.3	183	5.9	148
VS-RTSA-50-1140-07-AB-FP	29.9	748	23.5	588	18.6	465	15.6	390	12.5	313	10.0	250	8.0	200	6.5	163
VS-RTSA-50-1360-03-AB-FP	69.2	1730	55	1375	44.2	1105	26.2	655	21.1	528	17.0	425	14.0	350	11.4	285
VS-RTSA-50-1360-07-AB-FP	50.4	1260	39.7	992	31.4	785	36.0	900	29.8	745	24.6	615	20.4	510	17.0	425
VS-RTSA-55-1136-0711-AB-FP	21.6	540	17.7	442	14.7	367	N/A	N/A								
VS-RTSA-55-1246-0711-AB-FP	32.2	805	25.9	647	21.1	527	N/A	N/A								
VS-RTSA-55-1352-0507-AB-FP	43.8	1095	35	875	28.6	715	N/A	N/A								
VS-RTSA-60-1240-0707-AB-FP	25.9	647	20.7	517	16.8	420	N/A	N/A								
VS-RTSA-60-1345-0507-AB-FP	34	850	27.6	690	22.6	565	N/A	N/A								
VS-RTSA-60-1348-0711-AB-FP	30.1	752	24.5	612	20.2	505	N/A	N/A								
VS-RTSA-65-1343-0507-AB-FP	30.8	770	24.8	620	20.4	510	N/A	N/A								
VS-RTSA-65-1343-0707-AB-FP	27.3	682	22	550	17.9	447	N/A	N/A								
VS-RTSA-70-1336-0507-AB-FP	23.6	590	19.2	480	15.8	395	N/A	N/A								
VS-RTSA-70-1336-0707-AB-FP	20.6	515	16.7	417	13.7	342	N/A	N/A								

<sup>1.</sup> The total combined EPA and weight of all fixtures, brackets, and other attachments mounting to a light pole cannot exceed the EPA and weight rating for a specified pole.

<sup>2.</sup> Standard EPA (Effective Projected Area) and weight values are based around standard commercial criteria (with 1.3 second gust factor) and AASHTO standards. Specific light pole design standards are available from factory.

Custom products, configurations, options, and accessories available from factory.

<sup>4.</sup> Satisfactory performance of light poles is dependent upon the structure being properly attached to a supporting foundation of adequate design.

<sup>&</sup>quot;+" indicates a vibration dampener is standard.



#### EPA Loading Guide (2017 Florida Building Code)

	110m	ph FBC	120mph FBC		130mph FBC		140mph FBC		150mph FBC		160mph FBC		170mph FBC		180mph FBC	
VS-RTSA-20-5931-11-AB-FP	Max EPA (sq ft)	Max Weight (lb)														
VS-RTSA-20-5931-11-AB-FP	16	400	13.5	338	11.5	288	9.9	248	8.5	213	7.5	188	6.5	163	5.8	145
VS-RTSA-20-6537-11-AB-FP	20	500	17	425	14.2	355	12.2	305	10.7	268	9.4	235	8.4	210	7.3	183
VS-RTSA-25-5924-11-AB-FP	10.5	263	9.2	130	8	200	6.7	168	5.7	143	4.8	120	4.2	105	3.5	88
VS-RTSA-25-7035-11-AB-FP	17	425	14	350	12	300	10.3	258	8.5	213	7.8	195	6.7	168	6	150
VS-RTSA-25-7035-07-AB-FP	23	575	20	500	17	425	14.5	363	12.8	320	11	275	9.5	238	8.5	213
VS-RTSA-30-6624-11-AB-FP	10	250	8.7	218	7.5	188	6.2	155	5.2	130	4.4	110	3.8	95	3.2	80
VS-RTSA-30-8038-11-AB-FP	17.2	430	14.5	363	12.3	308	10.5	263	9	225	8	200	6.7	168	5.7	143
VS-RTSA-30-8038-07-AB-FP	29	725	25.3	633	21.5	538	18.5	463	16	400	14	350	12.4	310	10.8	270
VS-RTSA-35-7324-11-AB-FP	9.7	243	8.4	210	7	175	6	150	5	125	4	100	3.3	83	2.8	70
VS-RTSA-35-8536-11-AB-FP	15	375	12.5	313	10.7	268	9.2	230	7.7	193	6.5	163	5.5	138	4.5	113
VS-RTSA-35-9546-11-AB-FP	19.4	485	16.5	413	14	350	11.3	283	9.4	235	8	200	6.5	163	5.3	133
VS-RTSA-39-7824-11-AB-FP	10	250	8.3	208	7	175	5.9	148	4.8	120	4	100	3.3	83	2.5	63
VS-RTSA-39-9036-11-AB-FP	14.4	360	12	300	10	250	8.5	213	7	175	6	150	5	125	4	100
VS-RTSA-39-9036-07-AB-FP	24.5	613	21	525	18.5	463	16	400	14	350	12	300	10.3	258	8.7	218
VS-RTSA-45-1037-11-AB-FP	12	300	9.5	238	7.7	193	6.3	158	5	125	4	100	3	75	2	50
VS-RTSA-45-1037-07-AB-FP	22.5	563	18.8	470	15.5	388	13	325	11.1	278	9.5	238	8	200	6.5	163
VS-RTSA-45-1147-07-AB-FP	9	225	7	175	5.5	138	4	100	3.3	83	2.3	58	1.4	35	1	25
VS-RTSA-50-1030-11-AB-FP	18	450	15	375	12.5	313	10	250	8.6	215	7	175	5.5	138	4.8	120
VS-RTSA-50-1030-07-AB-FP	N/A	N/A														
VS-RTSA-50-1140-07-AB-FP	N/A	N/A														
VS-RTSA-50-1360-03-AB-FP	N/A	N/A														
VS-RTSA-50-1360-07-AB-FP	N/A	N/A														
VS-RTSA-55-1136-0711-AB-FP	N/A	N/A														
VS-RTSA-55-1246-0711-AB-FP	N/A	N/A														
VS-RTSA-55-1352-0507-AB-FP	N/A	N/A														
VS-RTSA-60-1240-0707-AB-FP	N/A	N/A														
VS-RTSA-60-1345-0507-AB-FP	N/A	N/A														
VS-RTSA-60-1348-0711-AB-FP	N/A	N/A														
VS-RTSA-65-1343-0507-AB-FP	N/A	N/A														
VS-RTSA-65-1343-0707-AB-FP	N/A	N/A														
VS-RTSA-70-1336-0507-AB-FP	N/A	N/A														
VS-RTSA-70-1336-0707-AB-FP	N/A	N/A														

<sup>1.</sup> The total combined EPA and weight of all fixtures, brackets, and other attachments mounting to a light pole cannot exceed the EPA and weight rating for a specified pole.

Standard EPA (Effective Projected Area) and weight values are based around Ultimate Wind Speed, Risk Category II, Exposure Category C. Specific light pole design standards are available from factory. Above data is based around the load centroid being at 2.5' above the pole top and with 2.0' eccentricity. Weight of horizontally eccentric load is capped at 100lb, all remaining weight mounted 2.5' above top of the pole.

<sup>3.</sup> Custom products, configurations, options, and accessories available from factory.

Satisfactory performance of light poles is dependent upon the structure being properly attached to a supporting foundation of adequate design.

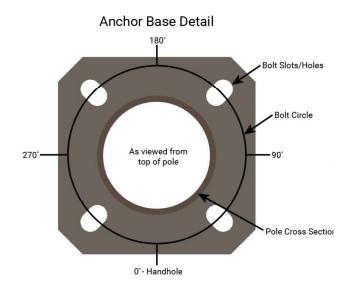
<sup>&</sup>quot;+" indicates a vibration dampener is standard.



#### Designation & Dimensional Information

		Pol	e Dimension	s		Base	Plate	Anchor Bolts		
Base Model	Nominal Mounting Height	Top OD (in)	Base OD (in)	Wall Thick (ga)	Structural Weight (lb)	Bolt Circle Diameter (in)	Sq (in) x Thick (in)	Dia x Length x Hook (in)	Projection (in)	
VS-RTSA-20-5931-11-AB-FP	20'-0"	3.1	5.9	11	140	8.5 - 9.5	10 x 0.875	1.00 x 36.00 x 4.00	3.88 - 4.38	
VS-RTSA-20-6537-11-AB-FP	20'-0"	3.7	6.5	11	160	9.0 - 10.0	10.5 x 0.875	1.00 x 36.00 x 4.00	3.88 - 4.38	
VS-RTSA-25-5924-11-AB-FP	25'-0"	2.4	5.9	11	155	8.5 - 9.5	10 x 0.875	1.00 x 36.00 x 4.00	3.88 - 4.38	
VS-RTSA-25-7035-11-AB-FP	25'-0"	3.5	7	11	200	9.5 - 10.5	10.88 x 0.875	1.00 x 36.00 x 4.00	3.88 - 4.38	
VS-RTSA-25-7035-07-AB-FP	25'-0"	3.5	7	7	280	9.5 - 10.5	10.88 x 1	1.00 x 36.00 x 4.00	4 - 4.5	
VS-RTSA-30-6624-11-AB-FP	30'-0"	2.4	6.6	11	200	9.0 - 10.0	10.5 x 0.875	1.00 x 36.00 x 4.00	3.88 - 4.38	
VS-RTSA-30-8038-11-AB-FP	30'-0"	3.8	8	11	265	10.5 - 11.5	11.5 x 0.875	1.00 x 36.00 x 4.00	3.88 - 4.38	
VS-RTSA-30-8038-07-AB-FP	30'-0"	3.8	8	7	380	10.5 - 11.5	11.5 x 1.25	1.25 x 42.00 x 6.00	4.75 - 5.25	
VS-RTSA-35-7324-11-AB-FP	35'-0"	2.4	7.3	11	250	10.0 - 11.0	11.25 x 0.875	1.00 x 36.00 x 4.00	3.88 - 4.38	
VS-RTSA-35-8536-11-AB-FP	35'-0"	3.6	8.5	11	315	11.0 - 12.0	12 x 1	1.00 x 36.00 x 4.00	4 - 4.5	
VS-RTSA-35-9546-11-AB-FP	35'-0"	4.6	9.5	11	370	12.5 - 13.5	13 x 1	1.00 x 36.00 x 4.00	4 - 4.5	
VS-RTSA-39-7824-11-AB-FP	39'-0"	2.4	7.82	11	285	10.5 - 11.5	11.5 x 0.875	1.00 x 36.00 x 4.00	3.88 - 4.38	
VS-RTSA-39-9036-11-AB-FP	39'-0"	3.58	9	11	355	12.0 - 13.0	12.38 x 1	1.00 x 36.00 x 4.00	4 - 4.5	
VS-RTSA-39-9036-07-AB-FP	39'-0"	3.58	9	7	515	12.0 - 13.0	12.38 x 1.25	1.25 x 42.00 x 6.00	4.75 - 5.25	
VS-RTSA-45-1037-11-AB-FP	45'-0"	3.7	10	11	450	13 - 14	14 x 1	1.00 x 36.00 x 4.00	4 - 4.5	
VS-RTSA-45-1037-07-AB-FP	45'-0"	3.7	10	7	650	13 - 14	14 x 1.25	1.25 x 42.00 x 6.00	4.75 - 5.25	
VS-RTSA-45-1147-07-AB-FP	45'-0"	4.7	11	7	780	14.5 - 15.5	16.5 x 1.5	1.25 x 42.00 x 6.00	5 - 5.5	
VS-RTSA-50-1030-11-AB-FP	50'-0"	3	10	11	475	13 - 14	14 x 1	1.00 x 36.00 x 4.00	4 - 4.5	
VS-RTSA-50-1030-07-AB-FP	50'-0"	3	10	7	680	13 - 14	14 x 1.25	1.25 x 42.00 x 6.00	4.75 - 5.25	
VS-RTSA-50-1140-07-AB-FP	50'-0"	4	11	7	812	14.5 - 15.5	16.5 x 1.5	1.25 x 42.00 x 6.00	5 - 5.5	
VS-RTSA-50-1360-03-AB-FP	50'-0"	6	13	3	1335	17.5	18.5 x 1.75	1.75 x 84.00 x 6.00	6.25 - 6.75	
VS-RTSA-50-1360-07-AB-FP	50'-0"	6	13	7	1020	17	18 x 1.5	1.50 x 54.00 x 6.00	5.5 - 6	
VS-RTSA-55-1136-0711-AB-FP	55'-0"	3.58	11	7 & 11	890	14.5 - 15.5	16.5 x 1.5	1.25 x 42.00 x 6.00	5 - 5.5	
VS-RTSA-55-1246-0711-AB-FP	55'-0"	4.55	12	7 & 11	975	16	17 x 1.5	1.50 x 54.00 x 6.00	5.5 - 6	
VS-RTSA-55-1352-0507-AB-FP	55'-0"	5.16	12.5	5 & 7	1225	16.5	17.5 x 1.5	1.50 x 54.00 x 6.00	5.5 - 6	
VS-RTSA-60-1240-0707-AB-FP	60'-0"	4.01	12	7&7	1060	16	17 x 1.5	1.50 x 54.00 x 6.00	5.5 - 6	
VS-RTSA-60-1345-0507-AB-FP	60'-0"	4.46	12.5	5 & 7	1275	16.5	17.5 x 1.5	1.50 x 54.00 x 6.00	5.5 - 6	
VS-RTSA-60-1348-0711-AB-FP	60'-0"	4.83	13	7 & 11	1075	17	18 x 1.5	1.50 x 54.00 x 6.00	5.5 - 6	
VS-RTSA-65-1343-0507-AB-FP	65'-0"	4.25	13	5 & 7	1400	17	18 x 1.5	1.50 x 54.00 x 6.00	5.5 - 6	
VS-RTSA-65-1343-0707-AB-FP	65'-0"	4.25	13	7 & 7	1200	17	18 x 1.5	1.50 x 54.00 x 6.00	5.5 - 6	
VS-RTSA-70-1336-0507-AB-FP	70'-0"	3.55	13	5 & 7	1440	17	18 x 1.5	1.50 x 54.00 x 6.00	5.5 - 6	
VS-RTSA-70-1336-0707-AB-FP	70'-0"	3.55	13	7&7	1270	17	18 x 1.5	1.50 x 54.00 x 6.00	5.5 - 6	

<sup>1.</sup> The total combined EPA and weight of all fixtures, brackets, and other attachments mounting to a light pole cannot exceed the EPA and weight rating for a specified pole.



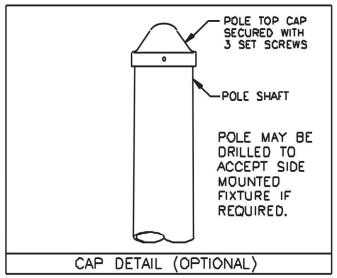
<sup>2.</sup> Custom products, configurations, options, and accessories available from factory.

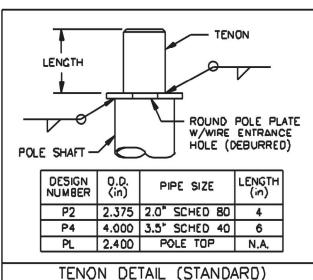
Satisfactory performance of light poles is dependent upon the structure being properly attached to a supporting foundation of adequate design.

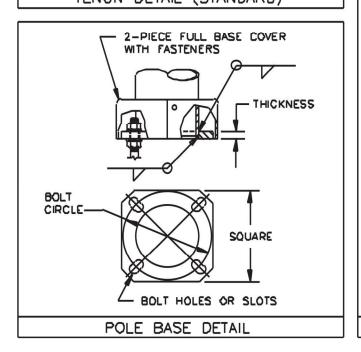
<sup>&</sup>quot;+" indicates a vibration dampener is standard.

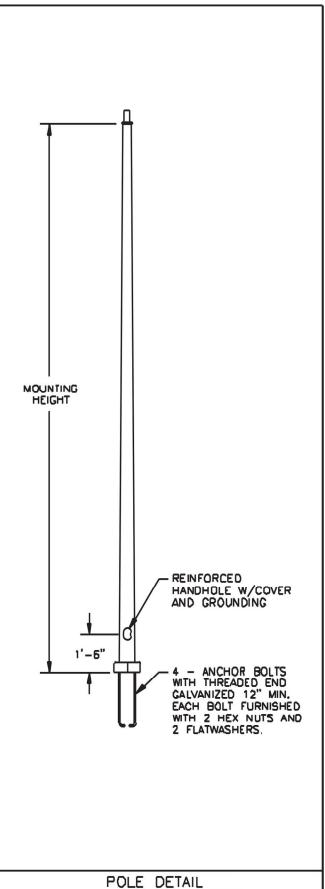


#### Dimensional Diagrams











#### Ordering Information

#### Ex: VS-RTSA-20-5931-11-AB-FP-MB-D2-FST

Product Family	Design	Length	Base OD	Top OD	Thickness	Anchor Bolts	Finish Type	Painted Color	Fixture Mounting
VS = NAFCO®	RTSA = Round Tapered Steel Anchor Base	10-30 = 10-30'	59 = 5.9"	31 = 3.1"	11 = 11ga	AB = Includes Anchor Bolts	GV = Galvanized Only (No Paint)	DB = Dark Bronze	PC = Cap Only, No Side Drilling
		C = Custom	65 = 6.5"	37 = 3.7"	7 = 7ga	LAB = Less Anchor Bolts	FP = Finish Painted	MB = Medium Bronze	PL = Open Top, No Cap or Side Drilling
			70 = 7"	24 = 2.4"	0711 - 7 & 11ga	C = Custom	FPGV – Finished Painted Over Galvanizing	BK = Black	D1 = Drill Single
			66 = 6.6"	35 = 3.5"	0507 = 5 & 7ga		C = Custom	WH = White	D2 = Drill 2@180
			80 = 8"	38 = 3.8"	0707 = 7 & 7ga			LG = Light Gray	D3 = Drill 3@120
			73 = 7.3"	36 = 3.6"	C = Custom			SG = Slate Gray	D4 = Drill 4@90
			85 = 8.5"	46 = 4.6"				DG = Dark Green	D5 = Drill 2@90
			95 = 9.5"	36 = 3.58"				SL = Silver	D6 = Drill 3@90
			78 = 7.82"	47 = 4.7"				RAL = Custom RAL Match	P1 = 4" OD x 5" Long Tenon
			90 = 9"	30 = 3"				C = Custom	P2 = 2.38" OD x 4" Long Tenon
			10 = 10"	40 = 4"					P3 = 3.50" OD x 6" Long Tenon
			11 = 11"	60 = 6"					P4 = 4" OD x 6" Long Tenon
			13 = 13"	46 = 4.55"					P5 = 2.88" OD x 4" Long Tenon
			12 = 12"	52 = 5.16"					P6 = 2.88" OD x 5" Long Tenon
			13 = 12.5"	40 = 4.01"					P7 = 2.38" OD x 5" Long Tenon
			C = Custom	48 = 4.83"					PQ = 2.38" OD x 12" Long Tenon
				45 = 4.46"					PD = 3" OD x 3" Long Tenon
				43 = 4.25"					P9 = Custom Size Tenon
				36 = 3.55"					
				C = Custom					

	Options & Accessories (Add as Suffix)	
Option	Option	Accessories
SPL = Special Cut Length (Please Specify)	ULHH = UL Compliant Hand Hole	STAMP = Engineering Services, Signed & Sealed Calcs
BCSPCL = Special Base Plate to Match Existing Bolt Circle (May Add to Production Lead Time, May Require Special Base Cover)	NECHH = NEC 410.30 Compliant Hand Hole & Cover	STAMPCA = Engineering Services, CA Signed & Sealed Calcs
VDA = Internal Vibration Dampener, Factory Installed	EHH = Additional Hand Hole Opening w/ Cover Assembly (Specify Pole Height & Orientation)	PRE100 = Pre-Ship Anchor Bolts - 1.0" x 36" x 4"
VDF = Internal Vibration Dampener, Field Installable	FST = Festoon Provision, Electrical by Others (Specify Pole Height & Orientation)	PRE125 = Pre-Ship Anchor Bolts - 1.25" x 42" x 6"
FBCP = ABS Plastic Full Base Cover	CPL = NPT Pipe Coupling (Specify Pole Height, Orientation, & NPT Size)	PRE150 = Pre-Ship Anchor Bolts - 1.5" x 54" x 6"
FBCS = Steel Full Base Cover		PRE175 = Pre-Ship Anchor Bolts - 1.75" x 84" x 6"
PXDX = Side Drill + Tenon w/ Additional Hand Hole (Specify Tenon OD & Length)		

- See previous pages for base model configurations. Consult factory or your sales rep for deviations from base models.
- Please consult factory or your sales representative to verify options and accessories will work with your light pole part number. Custom products, configurations, options, and accessories available from factory. 2. 3.



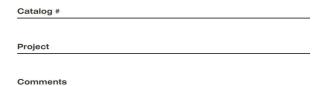
## NAFCO® RSSA ROUND STRAIGHT STEEL ANCHOR BASE LIGHT POLES











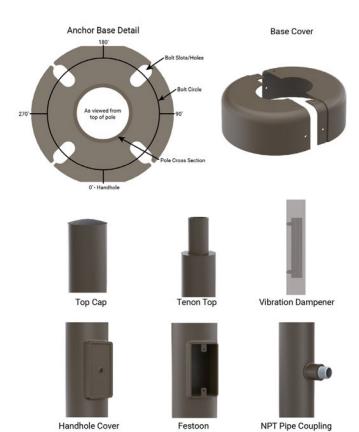


Proudly engineered and manufactured in the American Midwest - our NAFCO® family of professional-grade light pole products combines 50+ years of manufacturing expertise and top-notch Midwestern workmanship. Like all WiLL products, NAFCO® poles come supported by our unmatched design, engineering, and project support capabilities.

#### Specifications

- Pole Shaft The pole shaft is fabricated from hot rolled welded steel tubing of one-piece construction with a minimum yield strength of 42 KSI.
- Pole Top A removable top cap is provided for poles receiving drilling patterns for side-mount luminaire arm assemblies. Other pole top options include Tenon Top, Top Cap Only or Open Top which is typical when the pole top diameter matches the necessary slip-fit dimensions.
- Hand Hole A reinforced hand hole with grounding provision is provided at 1' from the base end of the pole assembly. Each hand hole includes a cover and cover attachment hardware. All pole assemblies are provided with a 2.5" x 5" rectangular hand hole (dimensions are nominal).
- Base Cover A two-piece full base cover fabricated from ABS plastic is provided with each pole assembly. Additional base cover options, including a cast aluminum and fabricated steel cover, are available upon request.
- Anchor Bolts Anchor bolts conform to ASTM F1554 Grade 55 and are provided with two hex nuts and two flat washers. Bolts have an "L" bend on one end and are galvanized a minimum of 12" on the threaded end.
- Hardware All structural fasteners are galvanized high strength carbon steel. All non-structural fasteners are galvanized or zinc-plated carbon steel or stainless steel.
- Finish Standard finishes are either Galvanized or Finish Painted. Additional finish options including Finish Paint over Galvanizing are available upon request.
- Design Criteria Standard EPA (Effective Projected Area) and weight values are based on Standard Commercial Criteria (with 1.3 gust factor) for side mounted fixtures only. Consult the factory on loading criteria for pole top mounted luminaires and/or brackets. Satisfactory performance of light poles is dependent upon the pole being properly attached to a supporting foundation of adequate design.







#### EPA Loading Guide (Commercial Design Criteria)

	70n w/ 1.3		80r w/ 1.3	nph 3 Gust	90n w/ 1.3		100i w/ 1.3		110 w/ 1.3		120 w/ 1.3		130 w/ 1.3		140r w/ 1.3		150 w/ 1.3	mph 3 Gust
Base Model	Max EPA (sq ft)	Max Weight (lb)																
VS-RSSA-10-3030-11-AB-FP	13.1	328	10	250	7.7	190	6	175	4.7	118	3.9	100	3.3	83	2.8	75	2.4	60
VS-RSSA-10-4040-11-AB-FP	25.1	630	19.1	480	15	375	12.2	305	9.9	275	8.3	225	7.0	200	6.0	175	5.2	150
VS-RSSA-10-4545-11-AB-FP	32.5	820	24.5	615	19.5	490	15.8	395	13.1	330	11.0	275	9.3	235	8.0	200	6.9	175
VS-RSSA-12-3030-11-AB-FP	10.4	260	7.7	195	5.8	145	4.4	130	3.4	90	2.8	75	2.3	75	1.9	75	1.6	75
VS-RSSA-12-4040-11-AB-FP	20.1	503	15	390	11.8	300	9.5	240	7.8	200	6.5	175	5.5	150	4.7	125	4.0	100
VS-RSSA-12-4545-11-AB-FP	26.2	655	19.8	495	15.7	395	12.7	320	10.5	263	8.7	220	7.4	200	6.3	175	5.5	150
VS-RSSA-14-3030-11-AB-FP	8.3	225	6	175	4.4	130	3.3	90	2.4	75	1.9	75	1.5	75	1.3	50	1.0	50
VS-RSSA-14-4040-11-AB-FP	16.5	425	12.2	305	9.4	250	7.6	195	6.2	175	5.1	150	4.3	125	3.6	100	3.1	100
VS-RSSA-14-4545-11-AB-FP	21.6	550	16.2	405	12.8	320	10.3	260	8.6	225	7.1	190	6.0	160	5.1	150	4.3	125
VS-RSSA-15-4040-11-AB-FP	16.5	425	9.6	250	7.4	185	5.9	150	6.2	175	5.1	150	4.3	125	3.6	100	3.1	100
VS-RSSA-15-4545-11-AB-FP	21.6	550	13.1	330	10.2	265	8.2	205	8.6	225	7.1	190	6.0	160	5.1	150	4.3	125
VS-RSSA-16-3030-11-AB-FP	6.5	175	4.6	125	3.2	100	2.3	60	1.5	50	1.2	45	0.9	45	0.7	45	0.5	45
VS-RSSA-16-4040-11-AB-FP	13.2	350	9.6	250	7.4	185	5.9	150	4.8	125	3.9	125	3.3	85	2.7	85	2.3	75
VS-RSSA-16-4545-11-AB-FP	17.5	440	13.1	330	10.2	265	8.2	205	6.7	175	5.6	150	4.7	125	4.0	120	3.4	100
VS-RSSA-18-3030-11-AB-FP	5.1	135	3.4	90	2.3	60	1.4	70	0.8	50	0.5	50	0.4	25	0.2	25	0.2	20
VS-RSSA-18-4040-11-AB-FP	10.7	275	7.6	190	5.7	180	4.5	130	3.6	100	3.0	75	2.4	75	2.0	75	1.7	50
VS-RSSA-18-4545-11-AB-FP	14.2	375	10.5	265	8.2	210	6.5	165	5.3	150	4.4	125	3.7	100	3.1	100	2.6	85
VS-RSSA-20-3030-11-AB-FP	3.9	115	2.4	100	1.4	75	N/A	N/A										
VS-RSSA-20-4040-11-AB-FP	8.6	225	6	150	4.4	150	3.4	125	2.7	75	2.2	70	1.7	70	1.4	50	1.2	45
VS-RSSA-20-4545-11-AB-FP	11.6	300	8.5	215	6.6	165	5.2	130	4.2	125	3.4	100	2.8	100	2.4	75	1.9	75
VS-RSSA-20-5050-11-AB-FP	15.4	400	11.7	300	9.1	230	7.2	180	5.9	150	4.8	125	4.0	125	3.2	100	2.6	100
VS-RSSA-25-4040-11-AB-FP	4.8	125	2.8	100	1.9	75	1.3	75	1.0	65	0.7	50	0.4	50	N/A	N/A	N/A	N/A
VS-RSSA-25-4545-11-AB-FP	6.9	175	4.8	130	3.6	90	2.7	90	2.1	100	1.6	100	1.3	75	1.0	75	0.7	50
VS-RSSA-25-5050-11-AB-FP	9.7	100	7.2	180	5.5	150	4.2	150	3.4	125	2.7	100	2.1	100	1.6	75	1.1	75
VS-RSSA-25-5050-07-AB-FP	16.0	400	12.1	300	9.4	250	7.4	200	6.0	150	5.0	125	4.1	110	3.2	100	2.5	100
VS-RSSA-30-4545-11-AB-FP	3.7	110	2.3	80	1.5	75	1	60	0.7	60	0.4	75	N/A	N/A	N/A	N/A	N/A	N/A
VS-RSSA-30-5050-11-AB-FP	6.0	125	4.2	150	3	125	2.2	100	1.7	100	1.3	65	0.8	65	0.4	50	N/A	N/A
VS-RSSA-30-5050-07-AB-FP	10.8	275	8	200	6.5	160	4.7	125	3.8	110	3.0	100	2.3	100	1.7	70	1.1	70

- 1. The total combined EPA and weight of all fixtures, brackets, and other attachments mounting to a light pole cannot exceed the EPA and weight rating for a specified pole
- 2. Standard EPA (Effective Projected Area) and weight values are based around standard commercial criteria (with 1.3 second gust factor) and AASHTO standards. Specific light pole design standards are available from factory.
- Custom products, configurations, options, and accessories available from factory.
- 4. Satisfactory performance of light poles is dependent upon the structure being properly attached to a supporting foundation of adequate design.

#### EPA Loading Guide (2017 Florida Building Code)

	110m	ph FBC	120m	ph FBC	130m	ph FBC	140m	ph FBC	150m	ph FBC	160m	ph FBC	170m	ph FBC	180m	ph FBC
Base Model	Max EPA (sq ft)	Max Weight (lb)														
VS-RSSA-10-3030-11-AB-FP	5.5	138	4.5	113	4	100	3.5	88	3	75	2.5	63	2	50	1.5	38
VS-RSSA-10-4040-11-AB-FP	12	300	10	250	8	200	7	175	6	150	5	125	5	125	4	100
VS-RSSA-10-4545-11-AB-FP	15.5	388	13	325	11	275	9	225	8.5	213	7	175	6.5	163	5.5	138
VS-RSSA-12-3030-11-AB-FP	4.5	113	3.5	88	3	75	2.5	63	2	50	1.5	38	1.5	38	1	25
VS-RSSA-12-4040-11-AB-FP	10	250	8	200	6.5	163	5.5	238	4.5	113	4	100	4	100	3.5	88
VS-RSSA-12-4545-11-AB-FP	13	325	10.5	263	9	225	7.5	188	7	175	6	150	5	125	4.5	113
VS-RSSA-14-3030-11-AB-FP	3.5	88	3	75	2.5	63	2	50	1.5	38	1	25	1	25	0.5	13
VS-RSSA-14-4040-11-AB-FP	9	225	7.5	188	6	150	5	125	4	100	3.5	88	3	75	3	75
VS-RSSA-14-4545-11-AB-FP	10.5	263	8.5	213	7	175	6	150	5.5	138	5	125	4	100	3.5	88
VS-RSSA-15-4040-11-AB-FP	9	225	7.5	188	6	150	5	125	4	100	3.5	88	3	75	3	75
VS-RSSA-15-4545-11-AB-FP	10.5	263	8.5	213	7	175	6	150	5.5	138	5	125	4	100	3.5	88
VS-RSSA-16-3030-11-AB-FP	3	75	2	50	1.5	38	1	25	1	25	0.5	13	0.5	13	N/A	N/A
VS-RSSA-16-4040-11-AB-FP	6.5	63	5	125	4	100	3	75	2.5	63	2	50	2	50	2	50
VS-RSSA-16-4545-11-AB-FP	9	225	7	175	5.5	138	4.5	113	4.5	113	4	100	3.5	88	3	75
VS-RSSA-18-3030-11-AB-FP	2	50	1.5	38	1	25	0.5	13	0.5	13	N/A	N/A	N/A	N/A	N/A	N/A
VS-RSSA-18-4040-11-AB-FP	5	125	4	100	3	75	2.5	63	1.5	38	1	25	1	25	1	25
VS-RSSA-18-4545-11-AB-FP	7	175	5.5	138	4.5	113	3.5	88	3.5	88	3	75	2.5	63	2	50
VS-RSSA-20-3030-11-AB-FP	1.5	38	1	25	0.5	13	N/A	N/A								
VS-RSSA-20-4040-11-AB-FP	4	100	3	75	2	50	1.5	38	1	25	0.5	13	0.5	13	0.5	13
VS-RSSA-20-4545-11-AB-FP	5.5	138	4.5	113	3.5	88	2.5	63	2.5	63	2.5	63	2	50	1.5	38
VS-RSSA-20-5050-11-AB-FP	7.5	188	6	150	5	125	5	125	4	100	3.5	88	2.5	63	2	50
VS-RSSA-25-4040-11-AB-FP	2	50	1	25	0.5	13	N/A	N/A								
VS-RSSA-25-4545-11-AB-FP	3	75	2	50	1	25	0.5	13	0.5	13	0.5	13	0.5	13	N/A	N/A
VS-RSSA-25-5050-11-AB-FP	4.5	113	3	75	3	75	3	75	2	50	1.5	38	1	25	0.5	13
VS-RSSA-25-5050-07-AB-FP	8	200	6	150	5.5	138	5	125	4	100	3.5	88	2.5	63	2	50
VS-RSSA-30-4545-11-AB-FP	1	25	N/A	N/A												
VS-RSSA-30-5050-11-AB-FP	2	50	0.5	13	N/A	N/A										
VS-RSSA-30-5050-07-AB-FP	4.5	113	3	75	3	75	3	75	2.5	63	1.5	38	1	25	0.5	13

<sup>1.</sup> The total combined EPA and weight of all fixtures, brackets, and other attachments mounting to a light pole cannot exceed the EPA and weight rating for a specified pole



<sup>&</sup>quot;+" indicates a vibration dampener is standard.

<sup>2.</sup> Standard EPA (Effective Projected Area) and weight values are based around Ultimate Wind Speed, Risk Category II, Exposure Category C. Specific light pole design standards are available from factory. Above data is based around the load centroid being at 2.5' above the pole top and with 2.0' eccentricity. Weight of horizontally eccentric load is capped at 100lb, all remaining weight mounted 2.5' above top of the pole.

<sup>3.</sup> Custom products, configurations, options, and accessories available from factory.

<sup>4.</sup> Satisfactory performance of light poles is dependent upon the structure being properly attached to a supporting foundation of adequate design.

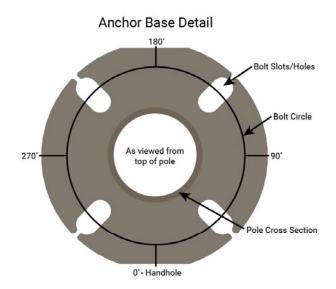
<sup>&</sup>quot;+" indicates a vibration dampener is standard.



#### Designation & Dimensional Information

		Pol	e Dimension	S		Base	Plate	Anchor Bolts			
Base Model	Nominal Mounting Height	Top OD (in)	Base OD (in)	Wall Thick (ga)	Structural Weight (lb)	Bolt Circle Diameter (in)	Sq (in) x Thick (in)	Dia x Length x Hook (in)	Projection (in)		
VS-RSSA-10-3030-11-AB-FP	10'-0"	3	3	11	55	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-10-4040-11-AB-FP	10'-0"	4	4	11	70	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-10-4545-11-AB-FP	10'-0"	4.5	4.5	11	75	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-12-3030-11-AB-FP	12'-0"	3	3	11	60	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-12-4040-11-AB-FP	12'-0"	4	4	11	80	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-12-4545-11-AB-FP	12'-0"	4.5	4.5	11	85	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-14-3030-11-AB-FP	14'-0"	3	3	11	70	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-14-4040-11-AB-FP	14'-0"	4	4	11	90	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-14-4545-11-AB-FP	14'-0"	4.5	4.5	11	95	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-15-4040-11-AB-FP	15'-0"	4	4	11	95	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-15-4545-11-AB-FP	15'-0"	4.5	4.5	11	100	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-16-3030-11-AB-FP	16'-0"	3	3	11	80	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-16-4040-11-AB-FP	16'-0"	4	4	11	100	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-16-4545-11-AB-FP	16'-0"	4.5	4.5	11	105	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-18-3030-11-AB-FP	18'-0"	3	3	11	90	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-18-4040-11-AB-FP	18'-0"	4	4	11	110	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-18-4545-11-AB-FP	18'-0"	4.5	4.5	11	115	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-20-3030-11-AB-FP	20'-0"	3	3	11	100	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-20-4040-11-AB-FP	20'-0"	4	4	11	120	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-20-4545-11-AB-FP	20'-0"	4.5	4.5	11	130	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-20-5050-11-AB-FP	20'-0"	5	5	11	145	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-25-4040-11-AB-FP	25'-0"	4	4	11	145	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-25-4545-11-AB-FP	25'-0"	4.5	4.5	11	155	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-25-5050-11-AB-FP	25'-0"	5	5	11	180	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-25-5050-07-AB-FP	25'-0"	5	5	7	260	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-30-4545-11-AB-FP	30'-0"	4.5	4.5	11	185	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-30-5050-11-AB-FP	30'-0"	5	5	11	210	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		
VS-RSSA-30-5050-07-AB-FP	30'-0"	5	5	7	305	7.0 - 9.0	10.5 x 0.75	0.75 x 17.00 x 3.00	3.25 - 3.75		

<sup>1.</sup> The total combined EPA and weight of all fixtures, brackets, and other attachments mounting to a light pole cannot exceed the EPA and weight rating for a specified pole.



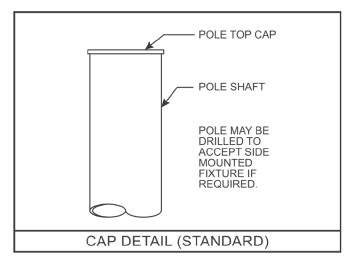
 $<sup>2. \</sup>quad \hbox{Custom products, configurations, options, and accessories available from factory}.$ 

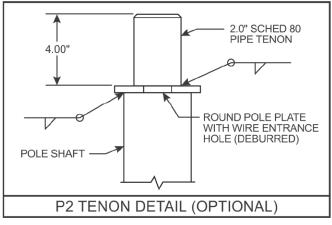
<sup>3.</sup> Satisfactory performance of light poles is dependent upon the structure being properly attached to a supporting foundation of adequate design.

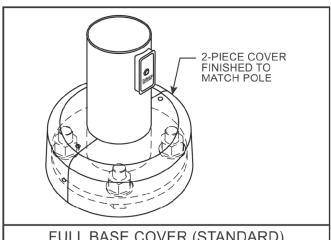
<sup>&</sup>quot;+" indicates a vibration dampener is standard.

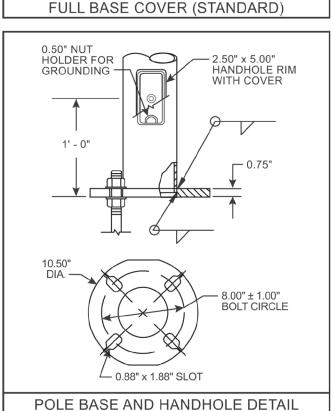


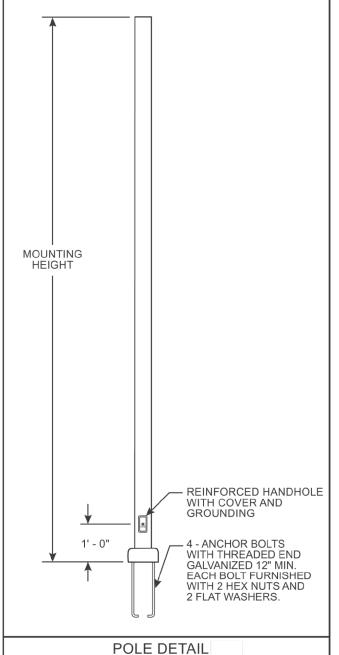
#### Dimensional Diagrams













#### Ordering Information

#### Ex: VS-RSSA-20-4040-11-AB-FP-MB-D2-FST

Product Family	Design	Length	Base OD	Top OD	Thickness	Anchor Bolts	Finish Type	Painted Color	Fixture Mounting
VS = NAFCO®	RSSA = Round Straight Steel Anchor Base	10-30 = 10-30'	30 = 3"	30 = 3"	11 = 11ga	AB = Includes Anchor Bolts	GV = Galvanized Only (No Paint)	DB = Dark Bronze	PC = Cap Only, No Side Drilling
		C = Custom	40 = 4"	40 = 4"	7 = 7ga	LAB = Less Anchor Bolts	FP = Finish Painted	MB = Medium Bronze	PL = Open Top, No Cap or Side Drilling
			45 = 4.5"	45 = 4.5"	C = Custom	C = Custom	FPGV – Finished Painted Over Galvanizing	BK = Black	D1 = Drill Single
			5 = 5"	5 = 5"			C = Custom	WH = White	D2 = Drill 2@180
			C = Custom	C = Custom				LG = Light Gray	D3 = Drill 3@120
								SG = Slate Gray	D4 = Drill 4@90
								DG = Dark Green	D5 = Drill 2@90
								SL = Silver	D6 = Drill 3@90
								RAL = Custom RAL Match	P1 = 4" OD x 5" Long Tenon
								C = Custom	P2 = 2.38" OD x 4" Long Tenon
									P3 = 3.50" OD x 6" Long Tenon
									P4 = 4" OD x 6" Long Tenon
									P5 = 2.88" OD x 4" Long Tenon
									P6 = 2.88" OD x 5" Long Tenon
									P7 = 2.38" OD x 5" Long Tenon
									PQ = 2.38" OD x 12" Long Tenon
									PD = 3" OD x 3" Long Tenon
									P9 = Custom Size Tenon

	Options & Accessories (Add as Suffix)	
Option	Option	Accessories
SPL = Special Cut Length (Please Specify)	ULHH = UL Compliant Hand Hole	STAMP = Engineering Services, Signed & Sealed Calcs
BCSPCL = Special Base Plate to Match Existing Bolt Circle (May Add to Production Lead Time, May Require Special Base Cover)	NECHH = NEC 410.30 Compliant Hand Hole & Cover	STAMPCA = Engineering Services, CA Signed & Sealed Calcs
VDA = Internal Vibration Dampener, Factory Installed	EHH = Additional Hand Hole Opening w/ Cover Assembly (Specify Pole Height & Orientation)	PRE075 = Pre-Ship Anchor Bolts - 0.75" x 17" x 3"
VDF = Internal Vibration Dampener, Field Installable	FST = Festoon Provision, Electrical by Others (Specify Pole Height & Orientation)	
FBCP = ABS Plastic Full Base Cover	CPL = NPT Pipe Coupling (Specify Pole Height, Orientation, & NPT Size)	
FBCS = Steel Full Base Cover		
PXDX = Side Drill + Tenon w/ Additional Hand Hole (Specify Tenon OD & Length)		

See previous pages for base model configurations. Consult factory or your sales rep for deviations from base models. Please consult factory or your sales representative to verify options and accessories will work with your light pole part number. Custom products, configurations, options, and accessories available from factory.



## NAFCO® STEEL BULLHORN BRACKETS, ROUND POLE MOUNT





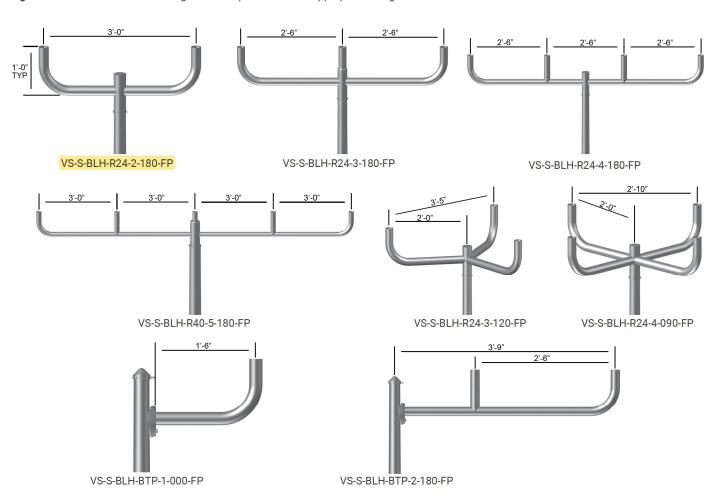


Catalog #			
Project			
Comments			

Proudly engineered and manufactured in the American Midwest - our NAFCO® family of professional-grade light pole products combines 50+ years of manufacturing expertise and top-notch Midwestern workmanship. Like all WiLL products, NAFCO® poles come supported by our unmatched design, engineering, and project support capabilities.

#### Specifications

- Center Hub (Excluding VS-S-BLH-R40-5-180-FP) The center hub is 2.88" OD x 0.203" wall with a minimum yield strength of 36,000 psi. The standard hub fits a 2.38" OD x 4" pole/tenon top. Other sizes available by contacting the factory.
- Center Hub (VS-S-BLH-R40-5-180-FP) The center hub is 4.50" OD x 0.188" wall with a minimum yield strength of 36,000 psi. The standard hub fits a 4" OD x 4" pole/tenon top. Other sizes available by contacting the factory.
- Arms Arms are 2.38" OD x 0.154" wall with a minimum yield strength of 36,000 psi. The maximum straight luminaire slipfit length is 4.63" for all arm brackets. For other tenon applications, please consult factory.
- Main Arm Only for VS-S-BLH-R24-5-180-FP is 2.38" OD x 0.218" wall with a minimum yield strength of 36,000 psi.
- Arm Attachment (VS-S-BLH-BTP-1-000-FP & VS-S-BLH-BTP-2-180-FP Only) Connection allows arm to be erected and held in place by gravity and secured by
- Hardware All structural fasteners are galvanized high strength carbon steel. All non-structural fasteners are galvanized or zinc-plated carbon steel or stainless
- Finish Standard finishes are either Galvanized or Finish Painted. Additional finish options including Finish Paint over Galvanizing are available upon request.
- Design Criteria Please reference Design Criteria Specification for appropriate design conditions.





#### Designation & Dimensional Information

		Max Qty of	Fixture	Max Luminaire	100mph v	v/ 1.3 Gust	Brack	et Size
Base Model	Mounting Type	Luminaires	Orientation	Spacing	Max Luminaire EPA (sq ft) <sup>1</sup>	Max Luminaire Weight (sq ft) <sup>1</sup>	EPA (sq ft)	Weight (lb)
VS-S-BLH-BTP-1-000-FP	Side Bolt	1	N/A	1'-6"	5	100	0.5	11
VS-S-BLH-BTP-2-180-FP	Side Bolt	2	180°	2'-6"	3	100	1.1	20
VS-S-BLH-R24-2-180-FP	Top Hub	2	180°	3'-0"	7.3	150	1	21
VS-S-BLH-R24-3-180-FP	Top Hub	3	180°	2'-6"	4.7	150	1.6	32
VS-S-BLH-R24-3-120-FP	Top Hub	3	120°	3'-5"	4.7	150	1.3	34
VS-S-BLH-R24-4-180-FP	Top Hub	4	180°	2'-6"	3.4	150	2.3	44
VS-S-BLH-R24-4-090-FP	Top Hub	4	90°	2'-10"	3.5	150	1.6	44
VS-S-BLH-R40-5-180-FP	Top Hub	5	180°	3'-0"	3	100	3.5	86

- 1. Maximum EPA (Effective Projected Area) and weight values are based on luminaires having a centroid 1'-0" above the bracket top and a maximum mounting height of 70'-0". Variations from sizes above are available upon inquiry at the factory.
- 2. Total combined weight and EPA of brackets and luminaires cannot exceed Design Information of specified pole.

Note: Additional sizes and configurations are available upon request.

#### Pole Top Bracket Attachment



**Bolt Attachment** 



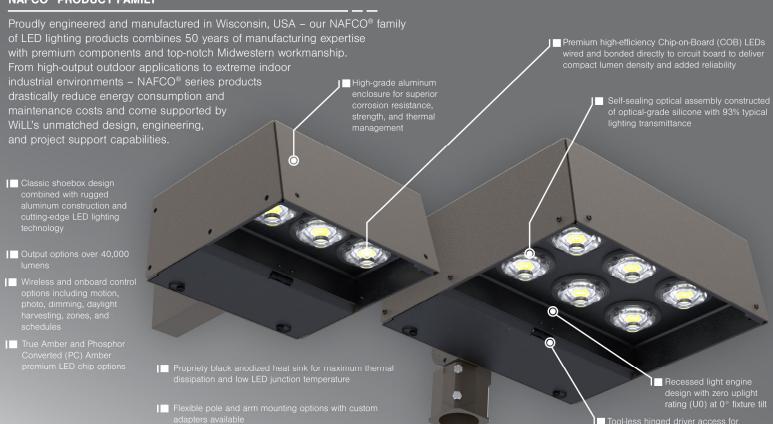
#### Ordering Information

#### Ex: VS-S-BLH-R24-4-090-FP

Product Family	Design	Mounting	Mounting Points	Orientation	Finish Type	Painted Color
VS = NAFCO®	S-BLH = Steel Bullhorn	BTP = Bolt Mount	1	000 = N/A	GV = Galvanized Only (No Paint)	DB = Dark Bronze
		R24 = 2.38" Round Pole/Tenon Top Mount	2	090 = 90°	FP = Finish Painted	MB = Medium Bronze
		R40 = 4" Round Pole/Tenon Top Mount (5 @180° ONLY)	3	180 = 180°	ГРGV = Гinished Painted Over Galvanizing	BK = Black
			4	120 = 120°	C = Custom	WH = White
			5			LG = Light Gray
						SG = Slate Gray
						DG = Dark Green
						SL = Silver
						RAL = Custom RAL Match
						C = Custom

- See above for base model configurations. Consult factory or your sales rep for deviations from base models. Please consult factory or your sales representative to verify options and accessories will work with your light pole part number. Custom products, configurations, options, and accessories available from factory.

#### **NAFCO® PRODUCT FAMILY**



# I NAFCO® SHX

# Area/Flood LED Lighting

Tool-less hinged driver access for easy install, technology upgrades, and maintenance





# NAFCO® SHX SHOEBOX AREA/FLOOD LED LIGHTING

















Project

Comments



#### Highlights

- Designed, engineered, and manufactured in Wisconsin, USA from premium domestic and imported components
- PPG® Commercial Performance Coatings custom color matching of RAL codes and architectural colors
- IES files, photometric reports, and lighting simulations available from factory design team
- · Output options over 40,000 lumens
- Easy driver and LED module access for technology upgrades and maintenance
- · Flexible mounting options with custom adapters available

#### Applications

- · General flood and area lighting
- Parking lots, ramps, walkways, and roadways
- Car dealerships, schools, and hospitals
- · Hotels and gas stations
- Retail stores and commercial buildings
- · Outdoor sports facilities including tennis courts
- · Amber and turtle applications
- RGB DMX color tuning applications

#### Construction & Finish

- Rugged aluminum chassis with excellent heat/impact resistance and hinged electrical access
- · Architectural grade powder coat enclosure and black anodized heat sink
- High-grade stainless steel hardware for superior strength and corrosion resistance
- Driver components are fully encased in potting material for moisture and vibration resistance

#### Light Poles & Arms

- WiLL offers one of the most comprehensive light pole, bracket, and arm catalogs in the industry
- · Aluminum, steel, fiberglass, and concrete materials
- · Straight, tapered, and decorative designs
- · Custom fabrication, finishing, and accessories available
- · Dedicated light pole application support team

#### Compliance & Warranty

- ETL Certification for UL STD 1598 & CSA STD C22.2 # 250.0 for wet locations
- Meets Buy American Act requirements
- · Standard 5-year limited warranty with extended factory warranties available
- Turtle and wildlife compliance options (consult factory)

#### Light Engine & Electrical

- Premium high-efficiency Chip-on-Board (COB) LEDs wired and bonded directly to circuit board to deliver compact lumen density and added reliability
- Self-sealing optical assembly constructed of optical-grade silicone with 93% typical lighting transmittance
- · -40°C to +45°C ambient operating temperature
- Standard AC input voltage of 120-277V 50/60 Hz; up to 480V available
- Isolated 1-10V PWM/3-timer-modes dimmable (standard) and dim-to-off with standby power  $\leq$  0.5W (optional)
- · Power factor of 0.90 min
- · Total harmonic distortion of 20% max
- Drivers include integral input Surge Protection of Differential Mode 6kV, Common Mode 10kV per EN 61000-4-5
- Thermally protected secondary 10kA surge suppression available (optional)
- · Always-on auxiliary power: 12VDC, 200mA (optional)
- Local specifying engineer recommended for product selection and local compliance
- · Licensed electrician required for installation

#### Control Options

- Integral passive infrared Bluetooth® sensor for motion, photo, dimming, and daylight harvesting control
- Synapse® wireless system for large-scale control of zones, dimming, schedules, and sensors
- DMX control options available from factory





#### EPA Chart

Base Model	0° Tilt	15° Tilt	30° Tilt	45° Tilt	60° Tilt	75° Tilt	90° Tilt
NF-SHS	0.6	0.7	0.9	1.0	1.1	1.1	1.4
NF-SHM	0.7	1.0	1.4	1.6	1.7	1.7	2.1

#### Specifications & Typical Lumen Output (WHITE LED)

19 Not 10 10	Weight	System	Engine	Drive	Typical HID		Distribution 3000K, 70 CRI			40	00K,	70 C	RI	50	00K,	70 C	CRI	57	700K	, 70 C	CRI		
Base Model	(lb)	Watts (W)	Qty	Current (A)	Replacement	Distribution	Lumens	В	U G	lm/W	Lumens	В	U G	lm/W	Lumens	В	U G	lm/W	Lumens	В	U G	i Im	n/W
						2 = Type II	6,491	2	0 2	158	6,611	2	0 2	161	6,731	2	0 2	164	6,731	2	0 2	2 1	64
						3 = Type III	6,562	2	0 2	160	6,684	2	0 2	163	6,805	2	0 2	166	6,805	2	0 2	2 1	66
						4 = Type IV	6,634	2	0 2	161	6,756	2	0 2	164	6,879	2	0 2	167	6,879	2	0 2	2 1	67
NF-SHS-40	12	41.1	1	0.8	100-150W	5W = 150° Type V Square	6,705	3	0 2	163	6,829	3	0 2	166	6,953	3	0 2	169	6,953	3	0 2	2 1	69
NF-3H3-40	12	41.1	'	0.6	100-13000	5M = 100° Type V Flood	6,634	3	0 1	161	6,756	3	0 1	164	6,879	3	0 1	167	6,879	3	0 1	1	67
						70 = 70° Type V Flood	6,919	3	0 1	168	7,047	_	0 1	171	7,175	_	0 1	175	7,175	3	_	_	75
						45 = 45° Medium Spot	6,491	3	0 2	158	6,611	3	0 2	161	6,731	3	0 2	164	6,731	3	0 2	2 1	64
						5N = 25° Narrow Spot	6,776	_	0 1	165	6,902	—	0 1	168	7,027	-	0 1	171	7,027	$\rightarrow$	0 1		71
						2 = Type II	12,983	_	0 3	158	13,223	—	0 3		13,463	—	0 3		13,463		0 3	_	64
						3 = Type III	13,126	_	0 3	160	13,368	-	0 3		13,611	_	0 3		13,611	$\rightarrow$	0 3	_	66
						4 = Type IV	13.268	3	0 3	162	13.514	—	0 3		13,759	-	0 3		13.759	$\rightarrow$	0 3	_	68
NF-SHS-80	13	82.1	2	0.8	150-250W	5W = 150° Type V Square	13,411	_	0 3	163	13,659	—	0 3		13,907	-	0 3	_	13,907	$\overline{}$	0 3		69
			_			5M = 100° Type V Flood	13,268	-	0 2	162	13,514	_	0 2		13,759	-	0 2		13,759	$\rightarrow$	0 2		68
						70 = 70° Type V Flood	13,839	-	0 1	169	14,095	-	0 1	172	14,351	_	0 1	170	14,351	4	0 1		75
						45 = 45° Medium Spot	12,983	_	0 3	158	13,223	-	0 3		13,463	-	0 3		13,463	_	0 3	_	64
						5N = 25° Narrow Spot	13,554	$\overline{}$	0 2	165	13,804	-	0 2		14,055	-	0 2		14,055	$\rightarrow$	0 2	_	71
						2 = Type II	19,474	$\overline{}$	0 3	158	19,834	-	0 3		20,194	-	0 3		20,194	$\rightarrow$	0 3	_	64
						3 = Type III	19,688	_	0 3	160	20,052		0 3		20,416		0 3		20,416		0 3	_	66
					220 400W	4 = Type IV	19,902	-	0 4	162	20,270		0 4		20,638	-	0 4	_	20,638	-	0 4	_	68
NF-SHS-120	IF-SHS-120 14 123.2 3	0.8	320-400W	5W = 150° Type V Square	20,116	4	0 3	163	20,488	-	0 3		20,860	-	0 3		20,860	4	0 3	_	69		
		_		020 10011	5M = 100° Type V Flood	19,902	_	0 2	162	20,270	$\rightarrow$	0 2		20,638	-	0 2		20,638	$\rightarrow$	0 2	_	68	
					70 = 70° Type V Flood	20,758	_	0 1	168	21,142	—	0 1	172	21,525	_	0 1	175	21,525	—	0 1	_	75	
						45 = 45° Medium Spot	19,474	$\overline{}$	0 3	158	19,834	_	0 3		20,194	_	0 3		20,194	$\rightarrow$	0 3	_	64
	-					5N = 25° Narrow Spot	20,330	_	0 3 0 4	165 158	20,706	-	0 3		21,082	-	0 3 0 4		21,082	5	0 3	_	71 64
						2 = Type II	25,965	-	-	_	26,445	$\rightarrow$	0 4 0 4		26,925	$\rightarrow$	-	_	26,925	$\rightarrow$	_	_	_
						3 = Type III 4 = Type IV	26,250 26,536	$\overline{}$	0 4	160 162	26,736 27,026	$\rightarrow$	0 4		27,221 27,517	_	0 4 0 4		27,221	$\rightarrow$	0 4	_	66 67
							26,821	-	0 4	163	27,026	$\rightarrow$	0 4		27,817	-	0 4		27,817	5	0 4	_	69
NF-SHM-160	20	164.3	4	0.8	400W+	5W = 150° Type V Square 5M = 100° Type V Flood	26.536	-	0 2	162	27,317	_	0 2		27,813	-	0 2		27,813	$\overline{}$	0 2	_	67
						70 = 70° Type V Flood	27,677	_	0 1	168	28,189	$\rightarrow$	0 2	172	28,700	$\vdash$	0 1	175	28,700	$\rightarrow$	0 1	_	75
						45 = 45° Medium Spot	25,965	-	0 4	158	26,445	—	0 4	_	26,925	-	0 4		26,925	$\overline{}$	0 4	_	64
						5N = 25° Narrow Spot	27,106	_	0 3	165	27,607	—	0 3	_	28,109	-	0 3		28,109	-	0 3	_	71
						2 = Type II	32,456	_	0 4	158	33,056	-	0 4		33,657	_	0 4		33,657	$\rightarrow$	0 4	_	64
						3 = Type III	32.813	$\overline{}$	0 4	160	33,419	$\rightarrow$	0 4		34.027	$\vdash$	0 4	_	34.027	4	_	_	66
						4 = Type IV	33.169	-	0 4	162	33,783	_	0 4	_	34,397	—	0 4		34,397	—	0 4	_	68
						5W = 150° Type V Square	33,526	5	0 4	163	34,146	-	0 4		34,767	-	0 4	_	34,767	5	0 4	_	69
NF-SHM-200	21	205.3	5	0.8	750W	5M = 100° Type V Flood	33,169	4	0 2	162	33,783	_	0 2		34,397	_	0 2		34,397	4	0 2		68
						70 = 70° Type V Flood	34,596	5	0 1	169	35,236	-	0 1	172	35,876	-	0 1	175	35,876	-	0 1	_	75
						45 = 45° Medium Spot	32,456	-	0 4	158	33,056	-	0 4	_	33,657	-	0 4		33,657	5	_	_	64
						5N = 25° Narrow Spot	33,883	-	0 3	165	34,509	-	0 3		35,136	-	0 3		35,136	5	0 3		71
						2 = Type II	38,948	_	0 4	158	39,668	_	0 4		40,388	—	0 4	_	40,388	-	0 4	_	64
						3 = Type III	39,376	_	0 5	160	40,104	-	0 5		40,832	-	0 5		40,832	-	0 5	_	66
		4 = Type IV	39,804	$\overline{}$	0 5	162	40,540	-	0 5		41,276	_	0 5		41,276	$\rightarrow$	0 5	_	68				
NE OUT		0.47		0.5	750 4	5W = 150° Type V Square	40,232	5	0 5	163	40,976	-	0 5		41,719	-	0 5		41,719	5	0 5	_	69
NF-SHM-250	22	246.4	6	0.8	750-1000W	5M = 100° Type V Flood	39,804	5	0 3	162	40,540	—	0 3		41,276	-	0 3		41,276	5	0 3		68
					1	70 = 70° Type V Flood	41,516	-	0 2	168	42,283	—	0 2	172	43,051	-	0 2		43,051	5	0 2	_	75
					1	45 = 45° Medium Spot	38,948	-	0 5	158	39,668	-	0 5		40,388	-	0 5		40,388	-	0 5	_	64
						5N = 25° Narrow Spot	40,660	5	0 3	165	41,412	5	0 3	168	42,163	5	0 3	171	42,163	5	0 3	1	71

Note: Typical lumen values are based on photometric tests performed in accordance with ANSI/IES LM-79-19. Actual performance may differ resulting from optical configuration, color temp and CRI, glare management, owner environment, and application.

Note: Data based on 25°C ambient operating temperature. Note: BUG ratings are calculated with fixture tilt set to 0°.

#### Specifications & Typical Lumen Output (WHITE LED)

	Base Model	Weight (lb)	System Watts (W)	Engine Qty	Drive Current (A)	LED Source	Lumens
	NF-SHS-CW/11-TA	14	41	3	0.25	True Amber (593 nm)	1,988
	NF-SHS-CW105-PCA	14	105	3	0.45	Phosphor Converted Amber (590 nm)	7,316
<b>\$</b>	NF-SHM-CW82-TA	22	82	6	0.25	True Amber (593 nm)	3,976
	NF-SHM-CW210-PCA	22	210	6	0.45	Phosphor Converted Amber (590 nm)	14,633





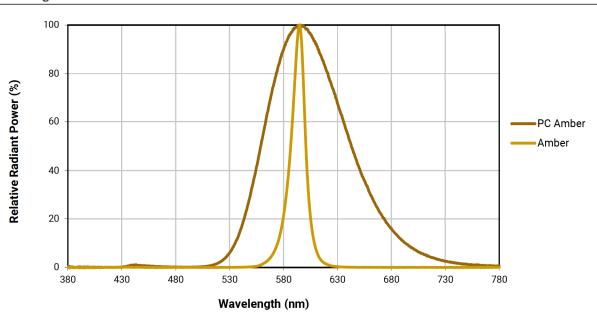
#### Lumen Multiplier & Maintenance (WHITE LED)

Ambient Temperature	Lumen Multiplier	TM-21 Lumen Maintenance (50,000 Hours)	Calculated L90 (hrs)	Calculated L70 (hrs)
0°C / 32°F	1.024	1.04	64,000	220,000
10°C / 50°F	1.021	1.02	64,000	220,000
25° C / 77°F	1.000	1.00	64,000	220,000
30°C / 86°F	0.993	0.99	58,000	194,000
35°C / 95°F	0.986	0.99	51,000	171,000
40° C / 104°F	0.979	0.98	45,000	151,000
45° C / 113°F	0.972	0.97	40,000	134,000

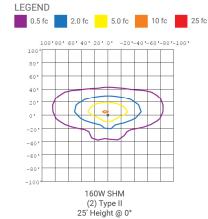
Current (A)						
Voltage	40W	80W	120W	160W	200W	250W
Input Current @ 120V (A)	0.34	0.68	1.03	1.37	1.71	2.05
Input Current @ 208V (A)	0.20	0.39	0.59	0.79	0.99	1.18
Input Current @ 240V (A)	0.17	0.34	0.51	0.68	0.86	1.03
Input Current @ 277V (A)	0.15	0.30	0.44	0.59	0.74	0.89
Input Current @ 347V (A)	0.12	0.24	0.36	0.47	0.59	0.71
Input Current @ 480V (A)	0.09	0.17	0.26	0.34	0.43	0.51

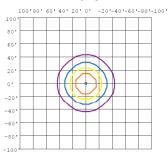
Note: Values calculated according to IESNA TM-21-11 methodology.

#### LED Chip Wavelengths



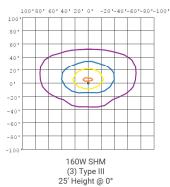
#### Photometric Diagrams

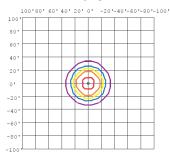




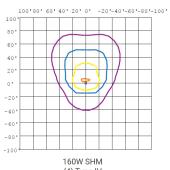
160W SHM (5M) 100° Type V Flood 25' Height @ 0°

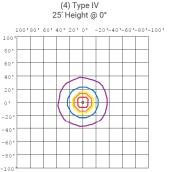
#### Simulated per IESNA LM-63-1995



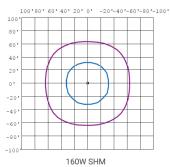


160W SHM (70) 70° Type V Flood 25' Height @ 0°

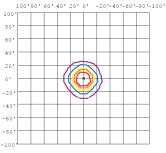




160W SHM (45) 45° Medium Spot 25' Height @ 0°



(5W) 150° Type V Square 25' Height @ 0°

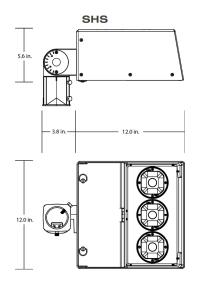


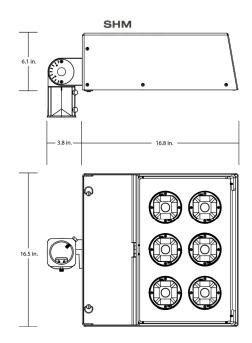
160W SHM (5N) 25° Narrow Spot 25' Height @ 0°



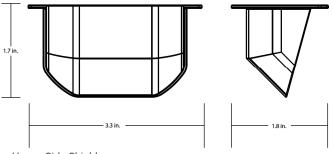


#### Dimensional Diagrams

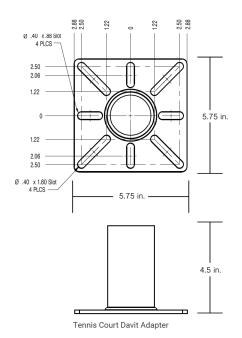


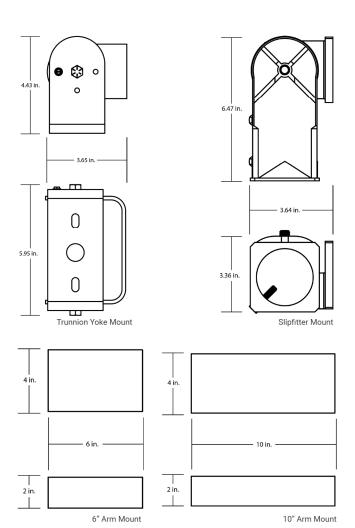


Note: Fixture diagrams shown with Slipfitter mount.



House Side Shield







#### Ordering Information

#### Ex: NF-SHM-250-50-MV-4-BZ-6S

Product Family	Design	Performance (Watts = Nominal Lumens)	Color Temp	Voltage	Distribution	Finish Color
NF = NAFCO®	SHS = 12" Shoebox Small Chassis	40 = 6,750	27 = 2700K, 70 CRI	MV = 120-277V	2 = Type II	BZ = Bronze (Default)
	SHM = 16" Shoebox Medium Chassis	80 = 13,500	30 = 3000K, 70 CRI	HV = 277-480V	3 = Type III	BK = Black
		120 = 20,250	40 = 4000K, 70 CRI	CV = Custom	4 = Type IV	WH = White
		160 = 27,000	50 = 5000K, 70 CRI		5W = 150° Type V Square	NA = Nat Alum Silver
		200 = 34,000	57 = 5700K, 70 CRI		5M = 100° Type V Flood	LG = Light Gray
		250 = 40,000	PCA = PC Amber (590 nm)		70 = 70° Type V Flood	SG = Slate Gray
		CW = Custom, Amber, & RGB	TA = True Amber (593 nm)		45 = 45° Medium Spot	DG = Dark Green
			CT = Custom		5N = 25° Narrow Spot	DP = Dark Platinum
					CD = Custom	GM = Graphite Metallic
						RAL = Custom RAL Match

	0	ptions & Accessories (Add as Suffix)	
Mounting	Option	Option	Accessories
6S = 6" Arm (Square Pole)	WHP3NP = 2' Cord w/o Plug, Stripped Pigtail	SRG27710 = 10kA Surge Suppressor (Field Replaceable), 120-277V	TLPC1 = Twist-Lock Photocell, 120-277V (Not Installed)
6R = 6" Arm (Round Pole)	WHP7NP = 6' Cord w/o Plug, Stripped Pigtail	SRG48010 = 10kA Surge Suppressor (Field Replaceable), 347-480V	TLPC4 = Twist-Lock Photocell, 347/480V (Not Installed)
10S = 10" Arm (Square Pole)	WHP11NP = 10' Cord w/o Plug, Stripped Pigtail	BPC1 = Button Photocontrol, 120-277V	HSS4-1/2/3/4/5/6 = House Side Shield Types I, II, III, & IV (Wattage Specific)
10R = 10" Arm (Round Pole)		BPC3 = Button Photocontrol, 347V	HSS5-1/2/3/4/5/6 = House Side Shield Type V (Wattage Specific)
SF = 2.38" OD Slipfitter		BPC4 = Button Photocontrol, 480V	TCAA = Tennis Court Davit Adapter (Not Installed)
TR = Trunnion Yoke		N5P = NEMA 5pin Twist-Lock Receptacle	AFW = Area/Flood Light Wall Bracket (Black Finish) (Not Installed)
CM = Custom		MPS = Programmable Motion Sensor w/ ON/OFF + Dimming + Photocontrol, Bluetooth Settings Adjustable, maximum coverage of 100' diameter from 40' mounting height	GFX = Wireless DMX Lighting Control System (Consult Factory)
		EB12FI = 1500 lm 90 min Emergency Battery Backup, 0°C to 40°C Ambient Operating Temp, 120- 277V Models Only (Consult Factory)	GFM = Wireless Mesh Lighting Control System (Consult Factory)
		EB12FIC = 1500 lm 90 min Cold Weather Emergency Battery Backup, -20°C to 40°C Ambient Operating Temp, 120-277V Models Only (Consult Factory)	

Note: Custom products, configurations, options, and accessories available from factory.



Arm Mounts







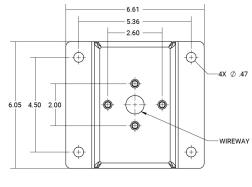
Slipfitter Mount

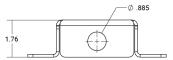
Trunnion Yoke Mount

Area/Flood Light Wall Bracket



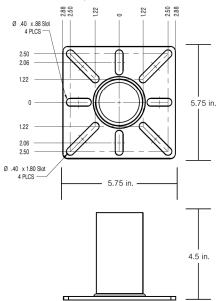
House Side Shield







Tennis Davit Adapter









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- (1) http://reliableplant.com/Read/30986/wisconsin-manufacturing-jobs
- (2) https://www.wmc.org/news/manufacturing-is-strong-in-wisconsin/
- (3) BizTimes Media, LLC (October 20, 2016)

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# TRAFFIC IMPACT STUDY FOR SKY TAVERN SKI AREA EXPANSION

May 8, 2023

#### PREPARED FOR:

**Robison Engineering Company, INC** 

**PREPARED BY:** 





#### YOUR QUESTIONS ANSWERED QUICKLY

#### Why did you perform this study?

This Traffic Impact Study evaluates the potential traffic impacts associated with the proposed Sky Tavern Ski Expansion Area project located adjacent to Mt. Rose Highway (SR 431), southwest of Reno, Nevada. This study was undertaken to determine the existing and future traffic conditions, quantify traffic volumes generated by the proposed project, identify potential impacts, and develop recommendations to mitigate impacts, if any are found.

#### What does the project consist of?

Sky Tavern is primarily a junior ski program facility including ski areas and a lodge. The proposed project is an expansion of the Sky Tavern Ski Area including an additional ski lift, the ski lodge expansion (approximately double in size) and additional parking. The site will include other amenities typically associated with summertime including a bicycle path, an airbag drop training and entertainment areas for special events such as weddings.

#### How much traffic will the project generate?

The study evaluates peaks hours for traffic entering and exiting the site on a typical Saturday during ski season. The highest hours occur in the morning arrival period and around noon. The project is anticipated to generate approximately 91 AM peak hour, and 43 Noon peak hour additional trips to the external roadway network.

#### How will project traffic affect the roadway network?

The study intersections are expected to operate at acceptable overall levels of service under Existing Plus Project and Future Year Plus Project conditions. A right turn lane from Mt. Rose Highway entering the site at Bum's Gulch Road is warranted based on the existing volumes; the addition of project traffic entering further justifies the need.

#### Are any improvements recommended?

Bum's Gulch Road serves as the primary entrance to the site, with the majority of traffic entering from the west via a right turn. It is recommended that an eastbound right-turn lane be constructed on Mt. Rose Highway for the intersection of Bum's Gulch Road / Mt. Rose Highway, based on NDOT's Access Management System and Standards. This will require a deviation letter since the spacing between Bums' Gulch Road and the Sky Tavern driveway does not allow for the full deceleration and taper length required. The deviation letter will be provided during the NDOT permitting process.



- It is recommended that the Sky Tavern Road approach at Mt. Rose Highway be striped to provide a separate left-turn and right-turn pockets. A stop bar and stop sign (36x36) should also be installed at this approach.
- It is recommended that the project install a stop bar / stop sign (36x36) on Bum's Gulch Road at Mt Rose Highway.
- Intersection ahead warning signs may be considered on Mt. Rose Highway as a safety improvement (see **Exhibit 3**).



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#### **LIST OF FIGURES**

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- 2. Project Site and Study Intersections
- 3. Preliminary Site Plan
- 4. Existing Traffic Volumes, Lane Configurations, & Controls
- 5. Project Trip Distribution & Assignment
- 6. Existing Plus Project Traffic Volumes, Lane Configurations, & Controls
- 7. Future Year Traffic Volumes, Lane Configurations, & Controls
- 8. Future Year Plus Project Traffic Volumes, Lane Configurations, & Controls

#### LIST OF APPENDICES

- A. Crash Data
- B. Traffic Count Data Sheets
- C. Existing LOS Calculations
- D. Existing Plus Project LOS Calculations
- E. Future Year LOS Calculations
- F. Future Year Plus Project LOS Calculations



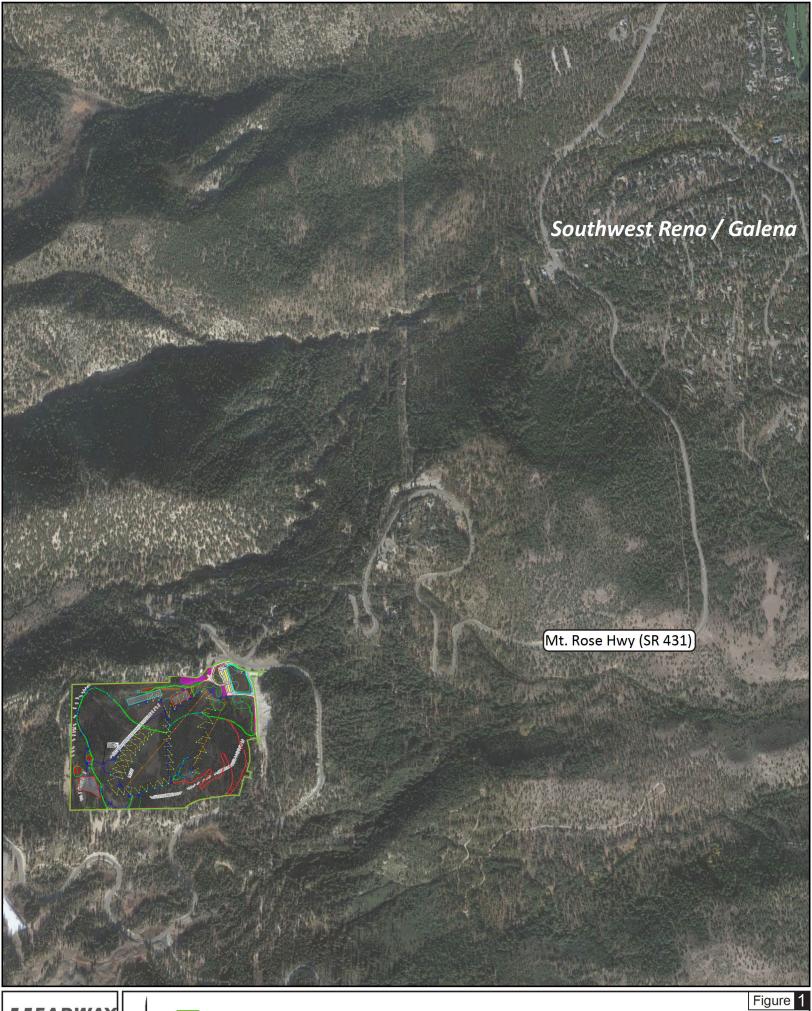
#### INTRODUCTION

This Traffic Impact Study (TIS) evaluates the potential traffic impacts associated with the proposed Sky Tavern Ski Area Expansion project. This study was undertaken to determine the existing and future (20-year horizon) traffic conditions, quantify traffic volumes generated by the proposed project, identify potential impacts, and develop recommendations to mitigate impacts, if any are found.

#### **Proposed Project Expansion**

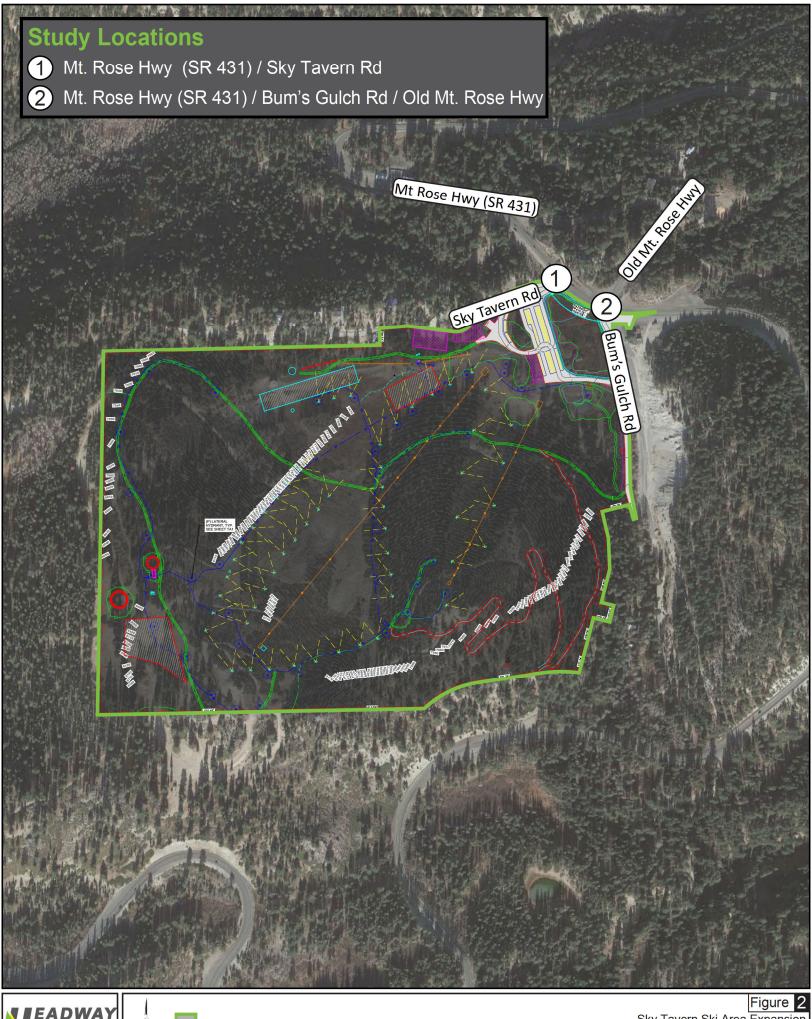
Sky tavern is a non-profit regional center owned by the City of Reno that provides summer and winter sports. Sky Tavern is primarily a junior ski program facility including ski areas and a lodge. The proposed project is an expansion of the Sky Tavern Ski Area including an additional ski lift, the ski lodge expansion (approximately double in size) and additional parking. The site will include other amenities typically associated with summertime including a bicycle path, an airbag drop training and entertainment areas for special events such as weddings. The site is located southwest of Reno NV, adjacent to Mt. Rose Highway (SR 431) at approximate mile marker (MM) 12.95 on parcel APN: (048-050-03). The regional project location can be seen in **Figure 1**, the project site location along with the study intersections are shown in **Figure 2**. A preliminary site plan is shown in **Figure 3**.





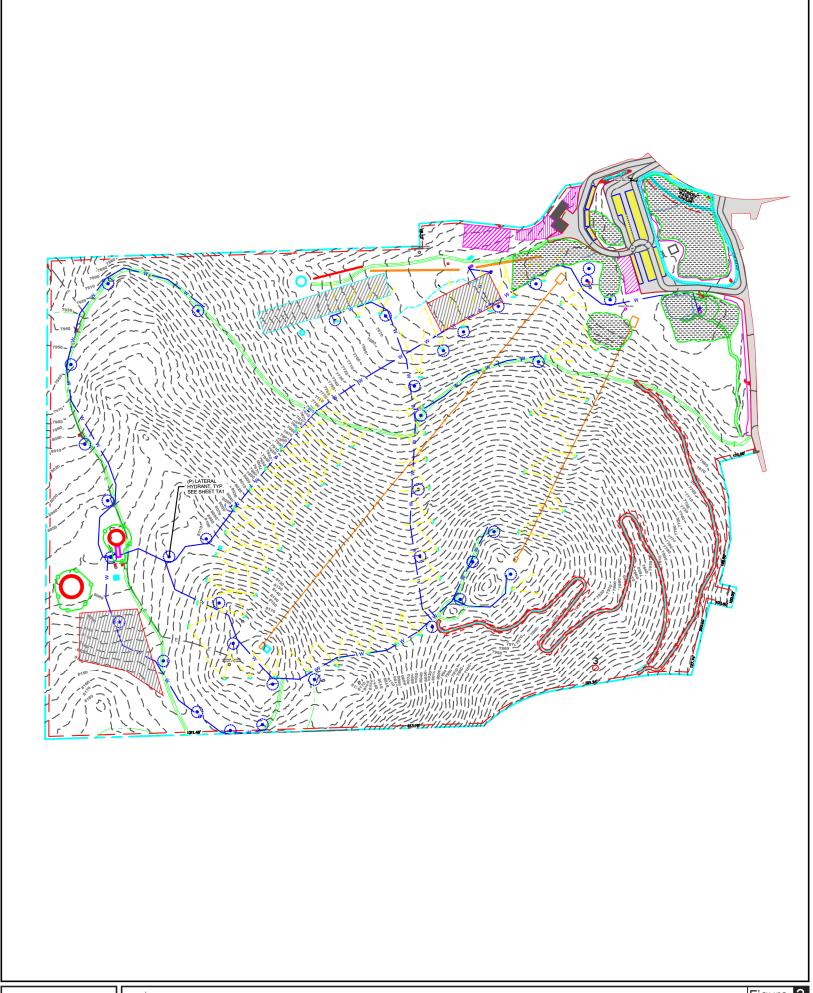














NO SCALE

#### Study Area and Evaluated Scenarios

The following intersections are included in this study:

- 1. Sky Tavern Road / Mt. Rose Highway (SR 431)
- 2. Bum's Gulch Road / Mt. Rose Highway (SR 431)

This study includes analysis for the typical peak ski season weekend (Saturday) AM and Noon hours as these are the periods of time in which peak traffic entering and exiting the site was observed to be the highest. The evaluated development scenarios are:

- Existing Conditions
- Existing Plus Project Conditions (representing opening year)
- Future Year Conditions (20-year horizon background) Conditions
- Future Year (20-year horizon background) Plus Project Conditions

#### **ANALYSIS METHODOLOGY**

Level of service (LOS) is a term commonly used by transportation practitioners to measure and describe the operational characteristics of intersections, roadway segments, and other facilities. This term equates seconds of delay per vehicle at intersections to letter grades "A" through "F" with "A" representing optimum conditions and "F" representing breakdown or over capacity flows.

#### Intersections

The complete methodology for intersection level of service analysis is established in *the Highway Capacity Manual (HCM), 6th Edition* published by the Transportation Research Board (TRB). **Table 1** presents the delay thresholds for each level of service grade at signalized and unsignalized intersections.

Table 1: Level of Service Definition for Intersections

Level of	Drief Description	Average Delay (seconds per vehicle)			
Service	Brief Description	Signalized Intersections	Unsignalized Intersections		
Α	Free flow conditions.	< 10	< 10		
В	Stable conditions with some affect from other vehicles.	10 to 20	10 to 15		
С	Stable conditions with significant affect from other vehicles.	20 to 35	15 to 25		
D	High density traffic conditions still with stable flow.	35 to 55	25 to 35		
E	At or near capacity flows.	55 to 80	35 to 50		
F	Over capacity conditions.	> 80	> 50		

Source: Highway Capacity Manual, 6th Edition



Level of service calculations were performed for the study intersections using the Synchro 11 software package with analysis and results reported in accordance with *HCM* methodology.

#### Level of Service Policy

#### **Regional Transportation Commission**

The Regional Transportation Commission's (RTC) 2050 Regional Transportation Plan (RTP) establishes level of service criteria for regional roadway facilities in the City of Reno, City of Sparks, and Washoe County. The current Level of Service policy is:

"All regional roadway facilities projected to carry less than 27,000 ADT at the latest RTP horizon — LOS D or better."

"All regional roadway facilities projected to carry 27,000 or more ADT at the latest RTP horizon – LOS E or better."

"All intersections shall be designed to provide a level of service consistent with maintaining the policy level of service of the intersecting corridors".

The roadways within the study area are projected to carry less than 27,000 ADT at the latest RTP horizon.

#### Nevada Department of Transportation

The Nevada Department of Transportation (NDOT) Traffic Impact Study Requirements publication states:

Level of service "C" will be the design objective for capacity and under no circumstances will less than level of service "D" be accepted for site and non-site traffic.

Hence, LOS "D" was used as the threshold criteria for this analysis.

Traffic engineering practitioners recognize that LOS E/F conditions for the side street approach, during the peak hour(s), does not indicate an intersection failure or the need for mitigation. This condition (LOS E/F for a minor side-street approach) commonly exists throughout urban and suburban areas and is manageable in most cases.

#### **EXISTING CONDITIONS**

#### **Roadway Facilities**

A brief description of the key roadways in the study area is provided below:



Mt. Rose Highway (SR 431) is classified as a minor arterial that runs southwest-northeast and connects to SR 28 and the junction of US 395A / Mt. Rose Highway. In the immediate project vicinity, Mt. Rose Highway has two lanes (one lane in each direction) and has a posted speed limit of 45 mph.

Bum's Gulch Road is a local road that runs north-south and connects to Mt. Rose Highway. The road primarily serves as the access to the Sky Tavern Ski area. In the immediate project vicinity, Bum's Gulch Road has two lanes (one lane in each direction) and has a speed limit of 25 mph. The roadway changes to Old Mt. Rose Highway north of Mt. Rose Highway.

Sky Tavern Road is a local road that is primarily an access to the Sky Tavern Ski area.

#### **Bicycle & Pedestrian Facilities**

There are no bicycle / pedestrian facilities (within project vicinity) on Mt. Rose Highway, Sky Tavern Road, or Bum's Gulch Road.

#### **Transit Facilities**

There are no transit facilities (within project vicinity) on Mt. Rose Highway, Sky Tavern Road, or Bum's Gulch Road.

#### **Crash History**

Vehicle crash data was obtained from NDOT and includes information from the 2016 to 2020 five-year period (the most current data available). As shown in **Exhibit 1**, twelve crashes occurred on Mt. Rose Highway within the project vicinity during the five-year period (some crashes overlap on the image). Four crashes resulted in injury and eight resulted in property damage. The injury crashes show vehicles running off the road / driving too fast for road conditions near the intersection of Mt. Rose Highway / Sky Tavern Road. The available crash data is found in **Appendix A**.



Exhibit 1: Crashes (2016-2020)

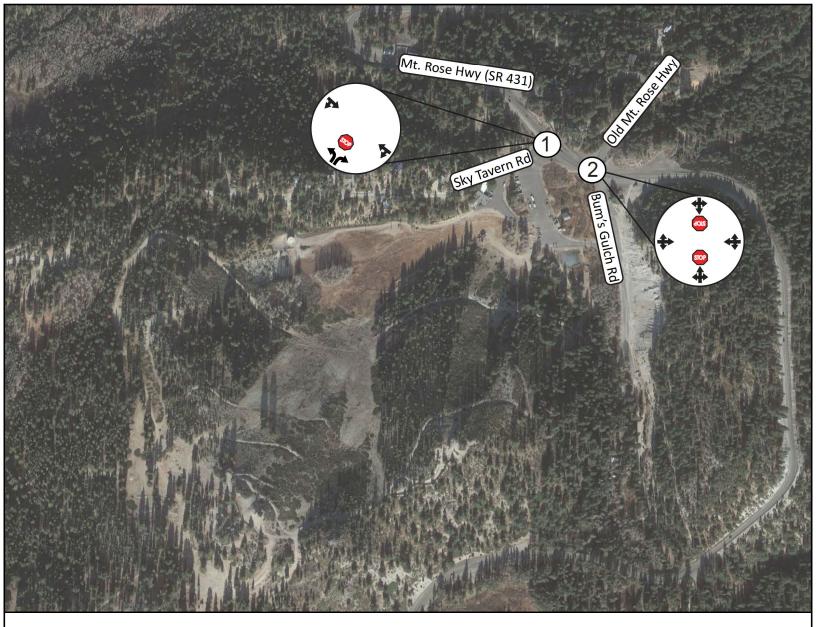
Mt. Rose Highway near the project site has horizontal curves and grades; these roadway features are associated with heighted safety concerns.



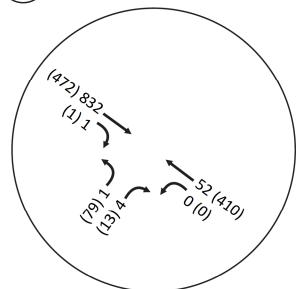
#### **Traffic Volumes**

Intersection turning movement counts were collected on a typical Saturday during peak ski season, on March 18, 2023, from 6:00 AM to 7:00 PM (13 hours). In general, a large number of vehicles arrived in the morning in a concentrated period (7:00-9:00) and departed throughout the day. Two peak hours were selected for analysis based on the highest volumes entering and exiting the ski area; these hours also corresponded with the highest volumes on Mt. Rose Highway. The selected peak hours represent AM arrival (7:15-8:15) and noon departure (11:15-12:15). Full count data is included in **Appendix B**. The existing AM and Noon peak hour intersection turning movements and mainline volumes are shown on **Figure 4**.

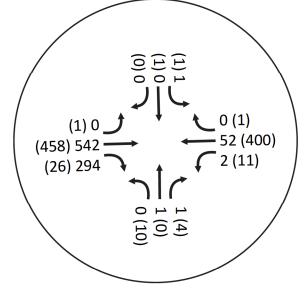








Mt. Rose Hwy (SR 431) / Bum's Gulch Rd / Old Mt. Rose Hwy











#### Intersection Level of Service Analysis

Existing weekend (Saturday) AM and Noon peak hour intersection level of service analysis was performed for the study intersections using Synchro 11 analysis software. The existing intersection lane configurations and controls are shown on **Figure 4. Table 2** shows the existing conditions level of service results and the technical calculations are provided in **Appendix C**.

**Table 2: Existing Intersection Level of Service** 

Int.	Intersection	Control AM		1	Noon	
ID	intersection	Control	Delay <sup>1</sup>	LOS	Delay <sup>1</sup>	LOS
	Sky Tavern Road / Mt. Rose Hwy					
	Overall	Side Street	0.1	Α	1.9	Α
1	NB Left		18.6	С	22.0	С
	NB Right	Stop	16.6	С	11.4	В
	WB Left		0.0	Α	0.0	Α
	Bum's Gulch Road / Old Mt.					
	Rose Hwy / Mt. Rose Hwy					
	Overall	Side Street	0.1	Α	0.4	Α
2	NB Approach	Stop	20.0	С	19.7	С
	SB Approach	3t0p	17.1	С	18.2	С
	EB Left		10.4	В	8.4	Α
	WB Left		0.0	Α	8.1	Α

Notes: 1. Delay is reported in seconds per vehicle for the overall intersection for signalized and all way stop controlled intersections, and for the worst approach/movement for side street stop-controlled intersections.

Source: Headway Transportation, 2023

As shown in **Table 2,** the existing study intersections currently operate within policy level of service thresholds during the AM and Noon peak hours.

#### PROJECT CONDITIONS

#### **Trip Generation**

Trip generation rates for the site improvements were developed by comparing:

- Rates calculated using the institute of Transportation Engineer's (ITE) *Trip Generation Manual, 11<sup>th</sup> Edition* for land use 466 "Snow Ski Area" based on the number of lifts. ITE describes Snow Ski Area: "A snow ski area typically includes chair lifts, ski runs, and lodge facility. A snow ski area may also contain equipment rental facilities, refreshments areas, locker rooms and small commercial/office spaces." It is noted that the ITE sample size is small; therefore, data is limited.
- The degree of modification of the site and amenities added relative to the existing counts entering and exiting.



**Table 3** shows the AM peak hour, and Noon peak hour ITE trip generation estimates compared to current site volumes entering and exiting. The ITE trips are lower than current counts; therefore, the trip generation was calculated as a percentage of current site traffic based on the expansion to provide a more accurate and conservative representation. The Sky Tavern Ski facility currently has two ski lifts and magic carpet lifts, the proposed expansion project will add an additional ski lift.

**Table 3: Trip Generation Comparison / Estimates** 

ITE Trip Generation Manual						
Land Use	Independent	Trips				
(ITE Code)	Variable		Saturday Peak	Hour of Ge	nerator	
(ITE Code)	Variable	AM	AM In/Out	Noon	Noon In/Out	
Snow Ski Area	3 Lifts (Represents Existing¹)	229	64 / 165	N/A	N/A	
(466)	1 Lift (Proposed)	76	21 / 55	N/A	N/A	
	l	Existing Trip	o Counts			
Fuiatina Counta Fu	hawina / Fritina	Saturday Peak Hour of Generator				
Existing Counts En	tering / Exiting	AM	AM In/Out	Noon	Noon In/Out	
		304	297 / 7	144	38 / 106	
	Calc	ulated Trip	Generation			
	Trips					
Calculated Trips (3	Calculated Trips (30% of existing entering/exiting traffic)		Saturday Peak	Hour of Ge	nerator	
entering/exiti			AM In/Out	Noon	Noon In/Out	
		91	89 / 2	43	11 / 32	

Notes: 1.

Estimated equivalent number of lifts for trip generation purposes currently on the site considering chair lifts and magic carpet lifts

**Table 3** shows that the ITE trip generation data underrepresents site traffic, with a significant variance on the in/out percentage. Therefore, the new trips to the site were estimated based on the modifications. One chair lift/magic carpet lift is being added (approximately 30% increase in ski lift facilities), along with ski lodge expansion, and additional parking. Other proposed site modifications are typically associated with summertime and do not impact the peak ski season trip generation. The existing counts into and out of the site were increased by 30% in order to accurately and conservatively represent the trip generation.

As shown in **Table 3**, the calculated trip generation rates are more conservative than the ITE manual trip generation rates. The proposed expansion project is expected to generate 91 AM peak hour, and 43 Noon peak hour additional trips.

#### **Trip Distribution**

Project trips were distributed to the adjacent roadway network based on existing traffic volumes and the proximity to the Reno areas. Project trips were distributed based on the following:



- > 85% to/from the west (Reno area) via Mt. Rose Highway
- 15% to/from the east via Mt. Rose Highway

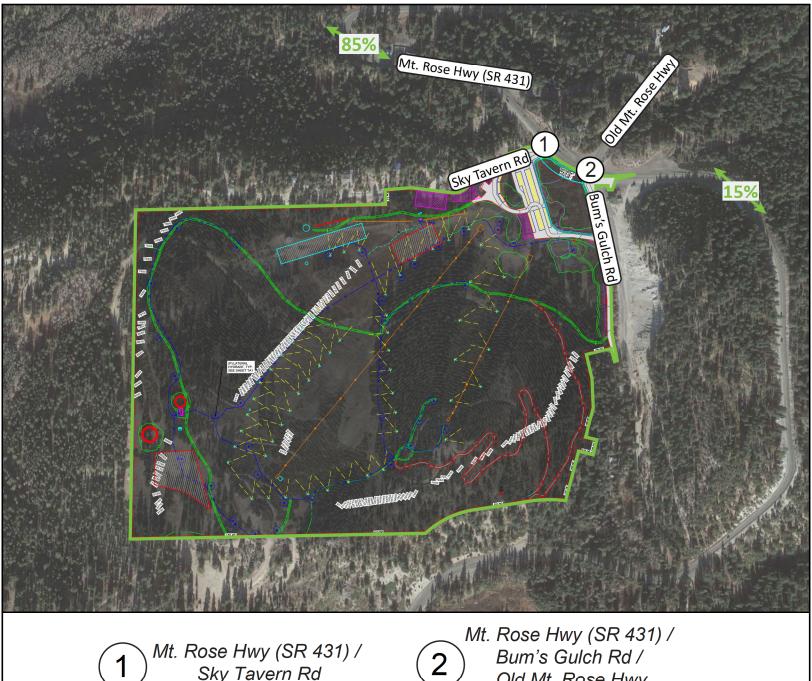
Figure 5 shows the project trip distribution and assignment.

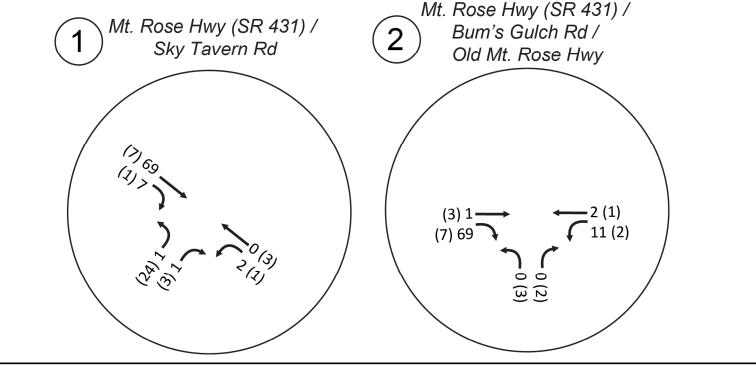
#### **Project Access and Access Management (Turn Lane Analysis)**

Two access points serve the Sky Tavern Ski area. During the ski season, the Bum's Gulch Road (Washoe County maintained) access serves as the primary entrance, and the Sky Tavern Road access serves as the primary exit.

NDOT's *AMSS* table 4-16 provides criteria for right-turn deceleration lanes at an intersection. An eastbound right-turn lane is currently warranted on Mt. Rose Highway at the intersection of Bum's Gulch Road / Mt. Rose Highway with over 250 vehicles per hour turning right in the morning. The addition of project traffic bolsters the need for a deceleration lane.









AM (Noon) Peak Hour Volumes **←**XX%**→** Trip Distribution



**Project Site** 

#### **EXISTING PLUS PROJECT CONDITIONS**

#### **Traffic Volumes**

Project trips (**Figure 5**) were added to the existing traffic volumes (**Figure 4**) to develop the Existing Plus Project conditions traffic volumes, shown on **Figure 6**.

#### Intersection Level of Service

AM and Noon peak hour intersection level of service analysis was performed for the study intersections based on the Existing Plus Project traffic volumes, the existing peak hour factors from the counts, and the lane configurations and controls shown on **Figure 6**. **Table 4** shows the level of service results, and the technical calculations are provided in **Appendix D**.

**Table 4: Existing Plus Project Level of Service** 

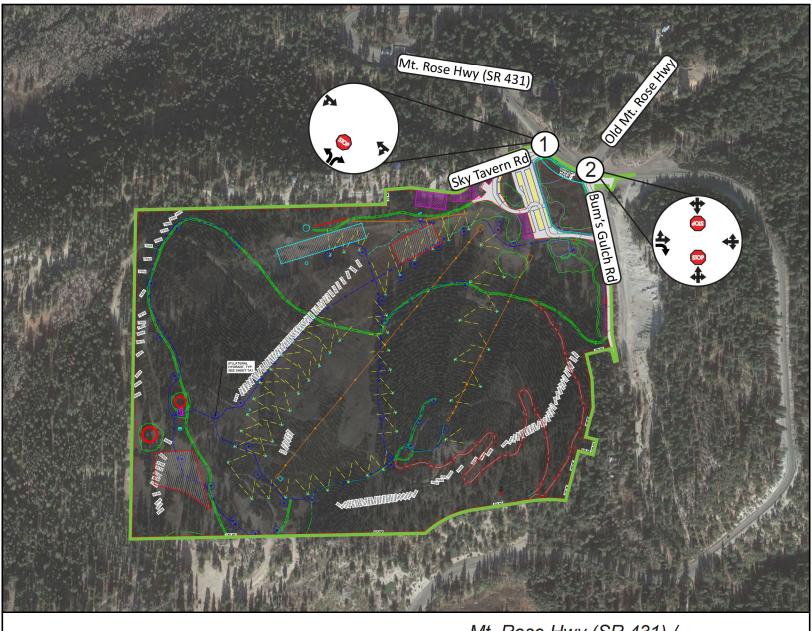
Int.	Intersection	Control	AN	1	No	Noon	
ID	intersection	Control	Delay <sup>1</sup>	LOS	Delay <sup>1</sup>	LOS	
	Sky Tavern Road / Mt. Rose Hwy						
	Overall	Side Street	0.2	Α	2.7	Α	
1	NB Left		20.5	С	24.5	С	
	NB Right	Stop	18.0	С	11.5	В	
	WB Left		10.4	В	8.4	Α	
	Bum's Gulch Road / Old Mt.						
	Rose Hwy / Mt. Rose Hwy						
	Overall	Side Street Stop	0.2	Α	0.6	Α	
2	NB Approach		22.1	С	20.0	С	
	SB Approach		18.2	С	18.3	С	
	EB Left		11.0	В	8.4	Α	
	WB Left		0.0	Α	8.4	Α	
	Additional ana	lysis with the in	stallation of a ri	ght-turn lane			
	Bum's Gulch Road / Old Mt.						
	Rose Hwy / Mt. Rose Hwy						
	Overall	Side Street	0.2	Α	0.5	Α	
2	NB Approach	Stop	22.1	С	20.0	С	
	SB Approach	Stop	14.7	В	17.9	С	
	EB Left		11.0	В	8.4	Α	
	WB Left		0.0	Α	8.1	Α	

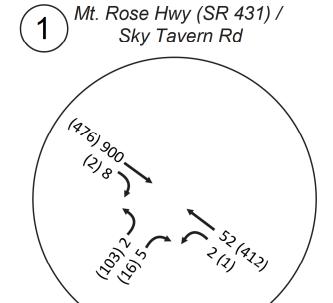
Notes: 1. Delay is reported in seconds per vehicle for the overall intersection for signalized and all way stop controlled intersections, and for the worst approach/movement for side street stop-controlled intersections.

Source: Headway Transportation, 2023

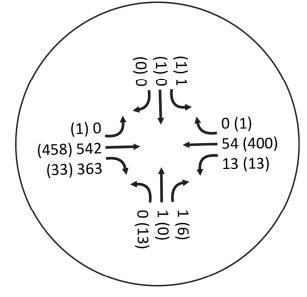
As shown in **Table 4**, the study intersections will operate within the level of service policy. The addition of the right-turn lane on Mt. Rose Highway for the intersection of Bum's Gulch Road is a safety improvement and is expected to slightly improve the operation.







Mt. Rose Hwy (SR 431) / Bum's Gulch Rd / Old Mt. Rose Hwy





AM (Noon) Peak Hour Volumes Stop Sign **Project Site** 

#### **FUTURE YEAR CONDITIONS**

The Future Year analysis estimates operating conditions for the 20-year horizon.

#### **Planned Roadway Improvements**

There are no planned roadway improvements within the study area.

#### **Traffic Volume Forecasts**

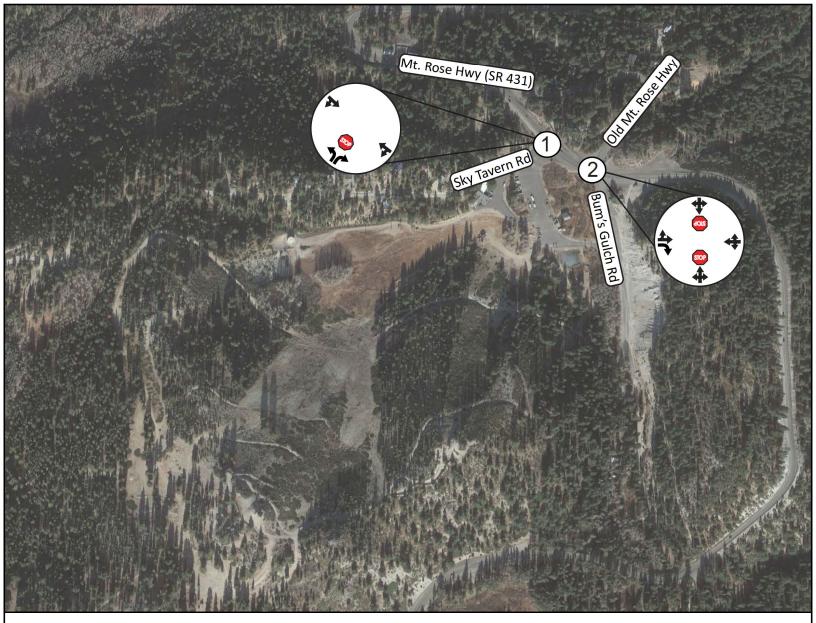
Future year (20-year horizon) traffic volumes were developed using an exponential annual growth rate to provide a baseline for assessing potential impacts on the future transportation system. The growth rate was developed using the RTC's regional travel demand model. **Table 5** shows the projected Annual Average Daily Traffic (AADT) volumes and growth rates from the RTC Traffic Demand Model.

**Table 5: RTC Model Growth Rates** 

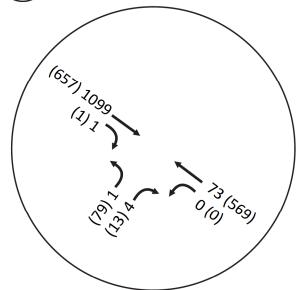
La castion .	Mt. Rose Hwy (SR 431)	Mt. Rose Hwy (SR 431)	Mt. Rose Hwy (SR 431)		
Location>	West of site	East of site	East of site		
	Travel	<b>Demand Model Vol</b>	umes		
2020 RTC Model	6,436	6,336	5,334		
2040 RTC Model	8,168	8,135	7,063		
2050 RTC Model	12,946	12,839	7,841		
Model Difference (2020-2040)	1,732	1,799	1,729		
Model Difference (2020-2050)	6,510	6,503	2,507		
	Growth Rate Calculation				
Compounded Growth rate (2020-2040)	1.20%	1.26%	1.41%		
Compounded Growth Rate (2020-2050)	2.36%	2.38%	1.29%		
Average growth Rate Per Year	1.65%	1.65%	1.65%		
20 Years Growth Factor	1.39	1.39	1.39		

As shown in **Table 5**, the growth rate varies in the study area, with an average rate of 1.65 percent per year. An exponential growth rate of 1.65 percent per year for 20 years was applied (growth factor of 1.39) to the existing through traffic volumes on Mt. Rose Highway to develop future year traffic volume forecasts at the study intersections. **Figure 7** shows the Future Year (20-year horizon) traffic volumes at the study intersections.

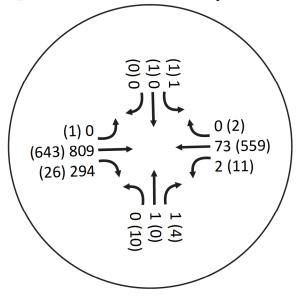








Mt. Rose Hwy (SR 431) /
Bum's Gulch Rd /
Old Mt. Rose Hwy







#### Intersection Level of Service

AM and Noon peak hour intersection level of service analysis was performed for the study intersections using Synchro analysis software. **Table 6** shows the Future Year conditions level of service results, and the technical calculations are provided in **Appendix E**.

Table 6: Future Year Intersection Level of Service

Int.	Intersection	Control	AN	1	No	on
ID	intersection	Control	Delay <sup>1</sup>	LOS	Delay <sup>1</sup>	LOS
	Sky Tavern Road / Mt. Rose Hwy					
	Overall	Cida Ctraat	0.1	Α	2.7	Α
1	NB Left	Side Street	26.6	D	42.1	E
	NB Right	Stop	22.6	С	13.4	В
	WB Left		0.0	Α	0.0	Α
	Bum's Gulch Road / Old Mt. Rose					
	Hwy / Mt. Rose Hwy					
	Overall	Side Street	0.1	0.5	Α	
2	NB Approach		31.7	D	30.4	D
	SB Approach	Stop	24.5	С	28.3	D
	EB Left		12.3	В	9.0	Α
	WB Left		0.0	Α	8.6	Α

Notes: 1. Delay is reported in seconds per vehicle for the overall intersection for signalized and all way stop controlled intersections, and for the worst approach/movement for side street stop-controlled intersections.

Source: Headway Transportation, 2023

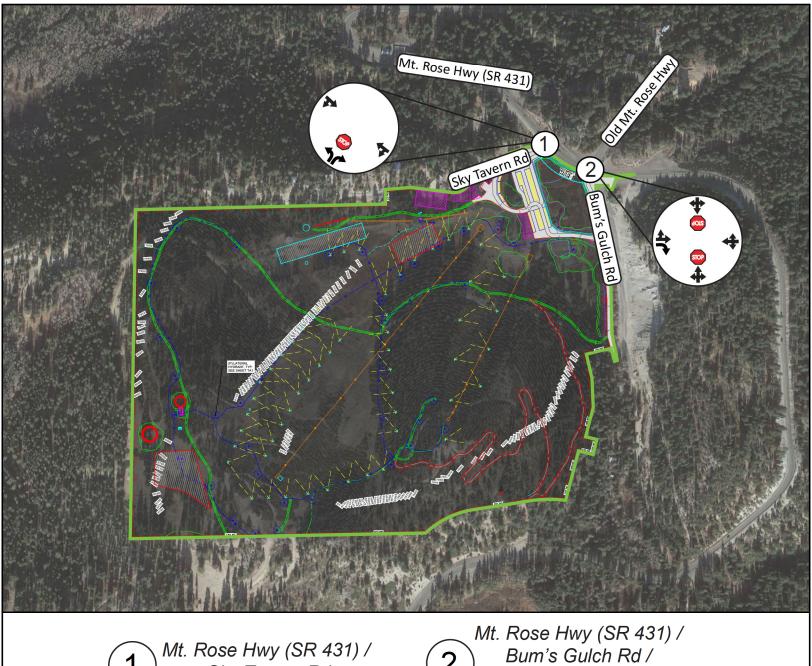
As shown in **Table 6**, the study intersections are expected to operate within the overall level of service policy. The northbound left-turn movement at the intersection of Sky Tavern Road / Mt. Rose Highway is expected to operate at a LOS E in the Noon peak hour without the expansion project. This is common for a side street approach, and reasonable for a Saturday during ski season.

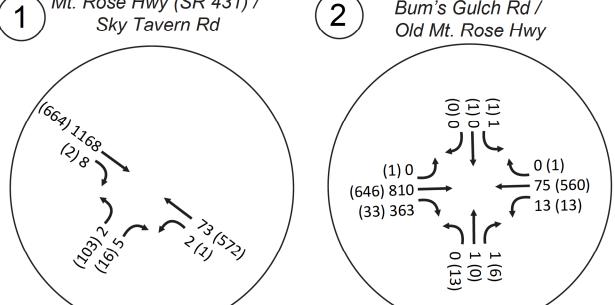
#### **FUTURE YEAR PLUS PROJECT CONDITIONS**

#### **Traffic Volumes**

Project trips (**Figure 5**) were added to the Future Year traffic volumes (**Figure 7**) to develop the Future Year Plus Project conditions traffic volumes, shown on **Figure 8**.









AM (Noon) Peak Hour Volumes Stop Sign **Project Site** 

#### Intersection Level of Service

AM and Noon peak hour intersection level of service analysis was performed for the study intersections based on the Future Year Plus Project traffic volumes. **Table 7** shows the level of service results, and the technical calculations are provided in **Appendix F**.

Table 7: Future Year Plus Project Intersection Level of Service

Int.	Intonocation	Control	AN	1	No	on
ID	Intersection	Control	Delay <sup>1</sup>	LOS	Delay <sup>1</sup>	LOS
	Sky Tavern Road / Mt. Rose Hwy					
	Overall	Side Street	0.2	Α	4.3	Α
1	NB Left	Stop	29.7	D	54.9	F
	NB Right	Stop	25.0	D	13.5	В
	WB Left		12.0	В	9.0	Α
	Bum's Gulch Road / Old Mt.					
	Rose Hwy / Mt. Rose Hwy					
	Overall	Side Street	0.2	Α	0.6	Α
2	NB Approach		36.1	E	31.1	D
	SB Approach	Stop	26.8	D	28.8	D
	EB Left		13.1	В	9.1	Α
	WB Left		0.0	Α	8.6	Α
	Additional ana	llysis with the in				
	Bum's Gulch Road / Old Mt.					
	Rose Hwy / Mt. Rose Hwy					
	Overall	Side Street	0.2	Α	0.6	Α
2	NB Approach		36.1	E	31.1	D
	SB Approach	Stop	20.8	С	28.1	D
	EB Left		13.1	В	9.1	Α
	WB Left		0.0	Α	8.6	Α

Notes: 1. Delay is reported in seconds per vehicle for the overall intersection for signalized and all way stop controlled intersections, and for the worst approach/movement for side street stop-controlled intersections.

Source: Headway Transportation, 2023

As shown in **Table 7**, the study intersections are expected to operate within the overall level of service policy. The northbound left-turn movement at the intersection of Sky Tavern Road / Mt. Rose Highway is expected to operate at a LOS F in the Noon peak hour. The northbound approach at the intersection of Bum's Gulch Road / Mt. Rose Highway is expected to operate at a LOS E in the AM peak hour. These LOS are common for side street approaches and are reasonable for a Saturday during ski season. The addition of the eastbound right-turn lane on Mt. Rose Highway for the intersection of Bum's Gulch Road is a safety improvement and is expected to slightly improve the operation.



#### **Project Access Turn Lane Lengths & Deviation Request**

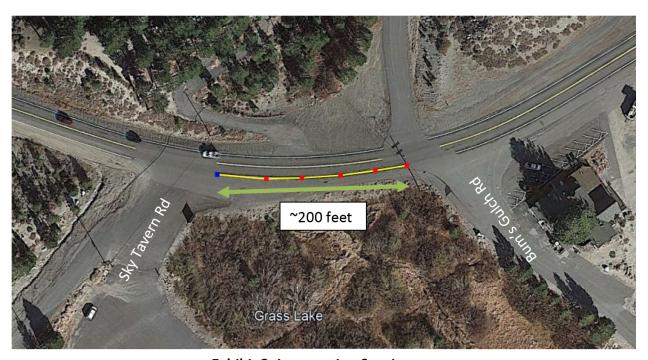
The NDOT standards provide minimum lengths for left-turn and right-turn deceleration lanes in Table 4-20. Deceleration lane lengths are determined based on the following:

Minimum Length of Deceleration Lane = Deceleration Length + Queue Storage Length

The deceleration length (including the taper length) for a 45-mph speed limit is 350 feet.

The eastbound right-turn is a free movement, therefore no storage length is needed.

**Exhibit 2** shows the intersection spacing. The eastbound right-turn deceleration lane should be constructed to provide the maximum length available, approximately 200 feet. This will require a deviation letter which is provided separately.



**Exhibit 2: Intersection Spacing** 



#### **SAFETY ENHANCEMENTS**

#### Potential Enhancements on Mt. Rose Highway

Mt. Rose Highway near the site access roadway has horizontal curves and grades, roadway features associated with heighted safety concerns. Additional signage may be considered as a safety improvement to proactively address safety and alert motorists to be aware of the upcoming intersections.

**Exhibit 3** show potential signage (either static or flashing) from the Manual on Uniform Traffic Control Devices (MUTCD):

- T-Intersection (W2-2) sign in the eastbound direction (west of Sky Tavern Road)
- Crossroad (W2-1) sign in the westbound direction (east of Bum's Gulch Road)



**Exhibit 3: MUTCD Signage.** 



#### **CONCLUSIONS & RECOMMENDATIONS**

The following is a list of our key findings and recommendations:

- The proposed project is an expansion of the Sky Tavern Ski Area including an additional ski lift, the ski lodge expansion (approximately double in size) and additional parking. The site will include other amenities typically associated with summertime including a bicycle path, an airbag drop training and entertainment areas for special events such as weddings.
- The study is conducted for a typical weekend (Saturday traffic) during ski season. The project is anticipated to generate 91 AM peak hour, and 43 Noon peak hour additional trips on the external roadway network.
- The study intersections currently operate at overall acceptable levels of service and are expected to continue to operate acceptably under Existing Plus Project conditions and Future Year Plus Project conditions. The northbound left-turn movement at the intersection of Sky Tavern Road / Mt. Rose Highway is expected to operate at a LOS F. The northbound approach at the intersection of Bum's Gulch Road / Mt. Rose Highway is expected to operate at a LOS E in the AM peak hour. These side street delays do not indicate an exceedance of LOS policy or warrant mitigations.
- It is recommended that an eastbound right-turn lane be constructed on Mt. Rose Highway for the intersection of Bum's Gulch Road / Mt. Rose Highway, based on NDOT's Access Management System and Standards. A deviation letter is provided separately from this report since the AMSS requirement of 350 feet cannot be met given the intersection spacing to Sky Tavern Road (approximately 200 feet).
- It is recommended that the following improvements be installed on Sky Tavern Road at the intersection with Mt. Rose Highway:
  - » Approach lane striping configuration (left-turn and right-turn pockets)
  - » Stop Bar
  - » Stop sign (36x36)
- It is recommended that the following improvements be installed on Bum's Gulch Road at the intersection with Mt. Rose Highway:
  - » Stop Bar
  - » Stop sign (36x36)
- Intersection approaching warning signs may be considered on Mt. Rose Highway as a safety improvement (see **Exhibit 3**).



# Appendix A NDOT Crash Data



Crash Severity	Crash Date	Crash Time	Primary Street	Dir	Secondary Street	Injured	Property Damage Only Injury Type Crash Type	Injury Type	Crash Type	Total Vehicles
INJURY CRASH	6/9/2016, 8:25 AM	3:25:00 PM	SR431	3	MILE MARKER 13	1	No Data	В	NON-COFFISION	1
PROPERTY DAMAGE ONLY   9/4/2016, 9:30 AM	9/4/2016, 9:30 AM	4:30:00 PM	SR431	W	SKY TAVERN RD	No Data	PDO	No Data	ANGLE	1
PROPERTY DAMAGE ONLY 11/1/2016, 2:55 PM	11/1/2016, 2:55 PM	9:55:00 PM	SR431	E	MILE MARKER 13	No Data	PDO	No Data	No Data NON-COLLISION	1
PROPERTY DAMAGE ONLY 12/23/2016, 4:38 AM	12/23/2016, 4:38 AM	12:38:00 PM	SR431	ш	MILE MARKER 13	No Data	PDO	No Data	HEAD-CN	2
PROPERTY DAMAGE ONLY 3/15/2017, 5:33 AM	3/15/2017, 5:33 AM	12:33:00 PM	SR431	Е	MILE MARKER 13	No Data	PDO	No Data	No Data NON-COLLISION	1
INJURY CRASH	1/19/2018, 3:27 AM	11:27:00 AM	SR431	E	MILE MARKER 13	1	No Data	C	NON-COFFISION	1
INJURY CRASH	6/30/2018, 4:28 PM	11:28:00 PM	SR431	E	SKY TAVERN RD	1	No Data	В	NON-COLLISION	1
PROPERTY DAMAGE ONLY	9/18/2018, 6:44 AM	1:44:00 PM	SR431	Е	MILE MARKER 13	No Data	PDO	No Data	NON-COFFISION	1
INJURY CRASH	1/20/2019, 3:45 AM	11:45:00 AM	SR431	E	MILE MARKER 13	1	No Data	В	ANGLE	2
PROPERTY DAMAGE ONLY   9/24/2019, 2:02 PM	9/24/2019, 2:02 PM	9:02:00 PM	SR431	E	MILE MARKER 13	No Data	PDO	No Data	ANGLE	2
PROPERTY DAMAGE ONLY 7/20/2020, 4:26 AM	7/20/2020, 4:26 AM	11:26:00 AM	SR431	E	SKY TAVERN RD	No Data	PDO	No Data	No Data SIDESWIPE, MEETING	2
PROPERTY DAMAGE ONLY 9/10/2020, 2:37 PM	9/10/2020, 2:37 PM	9:37:00 PM	SR431	ш	MILE MARKER 13	No Data	PDO	No Data	No Data NON-COLLISION	1

V1 Type	V1 Dir	V1 Driver Age	V1 Lane Num	V1 Action	V1 Driver Factors	V1 Driver Distracted	V1 Vehicle Factors
CARRY-ALL	Е	26	No Data	NOT REPORTED	APPARENTLY NORMAL	No Data	RAN OFF ROAD
PICKUP	W	33	1	GOING STRAIGHT	APPARENTLY NORMAL	No Data	No Data
SEDAN, 4 DOOR	M	92	No Data	GOING STRAIGHT	APPARENTLY NORMAL	No Data	DISREGARDED TRAFFIC SIGNS, SIGNALS, ROAD MARKINGS
HARDTOP, 4 DOOR	Е	21	1	GOING STRAIGHT	APPARENTLY NORMAL	No Data	DRIVING TOO FAST FOR CONDITIONS
SEDAN, 4 DOOR	M	54	1	GOING STRAIGHT	APPARENTLY NORMAL	No Data	No Data
UTILITY	Е	28	1	GOING STRAIGHT	APPARENTLY NORMAL	No Data	DRIVING TOO FAST FOR CONDITIONS
HATCHBACK/FASTBACK	Е	33	1	NOT REPORTED	APPARENTLY NORMAL	No Data	DRIVING TOO FAST FOR CONDITIONS
UTILITY	Е	65	1	NOT REPORTED	APPARENTLY NORMAL	No Data	FAILURE TO KEEP IN PROPER LANE OR RUNNING OFF ROAD
CARRY-ALL	No Data	39	1	GOING STRAIGHT	APPARENTLY NORMAL	No Data	DRIVING TOO FAST FOR CONDITIONS
STATION WAGON	Е	93	1	<b>NEGOTIATING A CURVE</b>	HAD BEEN DRINKING	No Data	OVER-CORRECTING/OVER-STEERING
VAN CAMPER	M	No Data	1	<b>NEGOTIATING A CURVE</b>	UNKNOWN	No Data	DROVE LEFT OF CENTER
SEDAN, 4 DOOR	Е	No Data	1	NEGOTIATING A CURVE	UNKNOWN	No Data	MECHANICAL DEFECTS

# Appendix B Traffic Count Data Sheets



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# Appendix C Existing LOS Calculations



Intersection							
Int Delay, s/veh	0.1						
		EDD	OFT	OED.	NIVAZI	NI\A/T	J
	EBL	EBR	SET	SER	NWL	NWT	
Lane Configurations	7	7	<b>}</b>		0	<b>ન</b>	J
Traffic Vol. veh/h	1	4	832	1	0	52	
Future Vol, veh/h	1	4	832	1	0	52 0	
Conflicting Peds, #/hr							
	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-		-	None	
Storage Length	0	0	-	-	-	-	
Veh in Median Storage, #		-	0	-	-	0	
Grade, %	0	- 07	0	- 07	- 07	0	
Peak Hour Factor	87	87	87	87	87	87	
Heavy Vehicles, %	0	0	1	0	0	1	
Mvmt Flow	1	5	956	1	0	60	
Major/Minor Mi	nor1	ı	Major1	N	Major2		
	1017	957	0	0	957	0	
Stage 1	957	_	_	_	_	_	
Stage 2	60	_	_	_	_	_	
Critical Hdwy	6.4	6.2	_	_	4.1	_	
Critical Hdwy Stg 1	5.4	-	_	_	-	_	
Critical Hdwy Stg 2	5.4	_	_	_	_	-/	
Follow-up Hdwy	3.5	3.3	_	_	2.2	-	
Pot Cap-1 Maneuver	266	315	_	_	727	_	
Stage 1	376	-	_	_	-	_	
Stage 2	968	_	_	_	_	_	
Platoon blocked, %	300		_	_		_	
Mov Cap-1 Maneuver	266	315	_	_	727	_	
Mov Cap-2 Maneuver	266	-	_		121	_	
Stage 1	376		_	_		-	
•	968		-	-			
Stage 2	900	-	-	_	-	-	
Approach	EB		SE		NW		
HCM Control Delay, s	17		0		0		
HCM LOS	С						
Minar Lana/Maiar Myrat		NIVA/I	NI\A/T	CDI 4 [	-DIO	OFT	
Minor Lane/Major Mvmt		NWL		EBLn1 E		SET	_
Capacity (veh/h)		727	-		315	-	
HCM Carter Dalay (a)		-		0.004		-	
HCM Control Delay (s)		0	-		16.6	-	
HCM Lane LOS HCM 95th %tile Q(veh)		A 0	-	C 0	C 0	-	
			_			_	

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	542	294	2	52	0	0	1	1	1	0	0
Future Vol, veh/h	0	542	294	2	52	0	0	1	1	1	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	79	79	79	79	79	79	79	79	79	79	79	79
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	0	686	372	3	66	0	0	1	1	1	0	0
Major/Minor M	/lajor1		ı	Major2			Minor1		N	Minor2		
Conflicting Flow All	66	0	0	1058	0	0	944	944	872	945	1130	66
Stage 1	-	-	-	-	-	-	872	872	-	72	72	-
Stage 2	_	_	_	_	_	_	72	72	_	873	1058	_
Critical Hdwy	4.1	_	_	4.1	_	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	- '	-	_	-	_	_	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	_	5 <b>-</b> 8	-	_	_	_	6.1	5.5	-	6.1	5.5	_
Follow-up Hdwy	2.2	-	_	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1549	-	-	666	-	-	244	264	353	244	205	1003
Stage 1	-	-	-	-	-	-	348	371	-	943	839	-
Stage 2	_	-	-	-	-	-	943	839	-	348	304	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1549	-	-	666	-	-	243	263	353	241	204	1003
Mov Cap-2 Maneuver	-	-	-	-	-	-	243	263	-	241	204	-
Stage 1	-	-	-	-	-	-	348	371	-	943	835	-
Stage 2	-	_	-	_	_	_	938	835	-	346	304	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.4			17.1			20		
HCM LOS							С			С		
Minor Lane/Major Mvmt	<u> </u>	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR:	SBL <sub>n1</sub>			
Capacity (veh/h)		301	1549	-	-	666	-	-	241			
HCM Lane V/C Ratio		0.008	-	-	-	0.004	-	-	0.005			
HCM Control Delay (s)		17.1	0	-	-	10.4	0	-	20			
HCM Lane LOS		С	Α	-	-	В	Α	-	С			
HCM 95th %tile Q(veh)		0	0	-	-	0	-	-	0			

Intersection						
Int Delay, s/veh	1.9					
		EDD	OFT	OEB	N IVA /I	NIVA/T
Movement	EBL	EBR	SET	SER	NWL	NWT
Lane Configurations	<b>*</b>	7	<b>1</b>		_	4
Traffic Vol, veh/h	79	13	472	1	0	410
Future Vol, veh/h	79	13	472	1	0	410
Conflicting Peds, #/hr	0	0	_ 0	0	0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	0	0	1	0	0	1
Mvmt Flow	84	14	502	1	0	436
Major/Minor N	1inor1	N	Major1	P	Major2	
Conflicting Flow All	939	503	0	0	503	0
Stage 1	503	-	-	-	-	-
Stage 2	436					
	6.4	6.2	-	-	4.1	-
Critical Hdwy			-	-		-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	295	573	-	-	1072	( <del></del> )
Stage 1	612	-	-	-	-	-
Stage 2	656	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	295	573	-	-	1072	-
Mov Cap-2 Maneuver	295	-	-	-	-	-
Stage 1	612	-	-	-	-	-
Stage 2	656	-	-	-	-	-
Approach	EB		SE		NW	
HCM Control Delay, s	20.5		0		0	
HCM LOS	20.3 C		U		U	
FIGNI LOS	U					
Minor Lane/Major Mvmt	<u> </u>	NWL	NWT	EBLn1 l	EBLn2	SET
Capacity (veh/h)		1072	-	295	573	-
HCM Lane V/C Ratio		-	-	0.285		-
HCM Control Delay (s)		0	-	22	11.4	-
HCM Lane LOS		A	-	С	В	-
HCM 95th %tile Q(veh)		0	-	1.1	0.1	_
, , , , , , , , , , , , , , , ,		•			<b>J</b> .,	

Intersection												
Int Delay, s/veh	0.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	458	26	11	400	1	10	0	4	1	1	0
Future Vol, veh/h	1	458	26	11	400	1	10	0	4	1	1	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	1	482	27	12	421	1	11	0	4	1	1	0
Major/Minor M	1ajor1		1	Major2		ľ	Minor1		N	Minor2		
Conflicting Flow All	422	0	0	509	0	0	944	944	496	946	957	422
Stage 1	-	-	-	-	-	-	498	498	-	446	446	-
Stage 2	-	-	-	-	-	-	446	446	-	500	511	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-		6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1148	-	-	1066	-		244	264	578	243	260	636
Stage 1	-	-	-	-	-	-	558	548	-	595	577	-
Stage 2	-	-	-	-	-	-	595	577	-	557	540	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1148	-	-	1066	-	: <b>-</b> :	240	260	578	238	256	636
Mov Cap-2 Maneuver	-	-	-	-	-	-	240	260	-	238	256	-
Stage 1	-	-	-	-	-	-	557	547	-	594	568	-
Stage 2	-	-	-	-	-	-	585	568	-	552	539	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.2			18.2			19.7		
HCM LOS							С			С		
Minor Lane/Major Mvmt		NBLn1	EBL	EBT	EBR	WBL	WBT	WBR:	SBLn1			
Capacity (veh/h)		288	1148	-	-	1066	-	-	247			
HCM Lane V/C Ratio		0.051		<u>-</u>		0.011	_		0.009			
HCM Control Delay (s)		18.2	8.1	0	_	8.4	0	_	19.7			
HCM Lane LOS		C	Α	A	_	Α.	A	_	C			
HCM 95th %tile Q(veh)		0.2	0	-	_	0	-	_	0			
		J.L										

# Appendix D Existing Plus Project LOS Calculations



Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBR	SET	SER	NWL	NWT
Lane Configurations	*	7	1>			4
Traffic Vol, veh/h	2	5	900	8	2	52
Future Vol, veh/h	2	5	900	8	2	52
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- -	None	-	None	-	None
Storage Length	0	0	_	-	-	-
Veh in Median Storage			0			0
	e, # 0	-	0	-	-	0
Grade, %	87	- 07	87	- 07	- 07	87
Peak Hour Factor		87		87	87	
Heavy Vehicles, %	0	0	1	0	0	1
Mvmt Flow	2	6	1034	9	2	60
Major/Minor	Minor1	N	Major1	N	//ajor2	
Conflicting Flow All	1103	1039	0		1043	0
Stage 1	1039	-	-	-	-	-
Stage 2	64	_	_	_	_	_
Critical Hdwy	6.4	6.2	_	<u>-</u>	4.1	<u>-</u>
Critical Hdwy Stg 1	5.4	0.2	_	-	4.1	-
	5.4					
Critical Hdwy Stg 2		- 2.2	-	-	- 2.2	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	236	283	=	-	675	1.5
Stage 1	344	-	-	-	-	-
Stage 2	964	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	235	283	-	-	675	-
Mov Cap-2 Maneuver	235	-	-	-	-	-
Stage 1	344	-	-	-	-	-
Stage 2	961	-	-	-	-	-
Annroach	EB		SE		NW	
Approach						
HCM Control Delay, s	18.7		0		0.4	
HCM LOS	С					
Minor Lane/Major Mvm	nt	NWL	NWT	EBLn1 E	EBLn2	SET
Capacity (veh/h)		675		235	283	
HCM Lane V/C Ratio		0.003	_	0.01	0.02	_
HCM Control Delay (s)		10.4	0	20.5	18	
HCM Lane LOS		В	A	20.5 C	C	_
HCM 95th %tile Q(veh)	\	0	-	0	0.1	-
	)	U	-	U	0.1	_

Intersection												
Int Delay, s/veh	0.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			44+			4			4	
Traffic Vol, veh/h	0	542	363	13	54	0	0	1	1	1	0	0
Future Vol, veh/h	0	542	363	13	54	0	0	1	1	1	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	79	79	79	79	79	79	79	79	79	79	79	79
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	0	686	459	16	68	0	0	1	1	1	0	0
Major/Minor N	/lajor1		1	Major2		ľ	Minor1		N	Minor2		
Conflicting Flow All	68	0	0	1145	0	0	1016	1016	916	1017	1245	68
Stage 1	-	-	-	-	-	-	916	916	-	100	100	-
Stage 2	-	-	-	-	-	-	100	100	-	917	1145	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	1-1	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1546	-	-	618	-		218	240	333	218	176	1001
Stage 1	-	-	-	-	-	-	329	354	-	911	816	-
Stage 2	-	-	-	-	-		911	816	-	329	277	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1546	-	-	618	-	-	213	234	333	212	171	1001
Mov Cap-2 Maneuver	-	-	-	-	-	-	213	234	-	212	171	-
Stage 1	-	-	-	-	-	-	329	354	-	911	794	-
Stage 2	-	-	-	-	-	-	886	794	-	327	277	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			2.1			18.2			22.1		
HCM LOS							С			С		
Minor Lane/Major Mvmt	<u> </u>	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR:	SBL <sub>n1</sub>			
Capacity (veh/h)		275	1546	-	-	618	-	-	212			
HCM Lane V/C Ratio		0.009	-	-	-	0.027	-	-	0.006			
HCM Control Delay (s)		18.2	0	-	-	11	0	-	22.1			
HCM Lane LOS		С	Α	-	-	В	Α	-	С			
HCM 95th %tile Q(veh)		0	0	-	-	0.1	-	-	0			

Intersection												
Int Delay, s/veh	0.2											
IIII Delay, S/VeII	0.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	7		4			4			4	
Traffic Vol, veh/h	0	542	363	13	54	0	0	1	1	1	0	0
Future Vol, veh/h	0	542	363	13	54	0	0	1	1	1	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	200	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	79	79	79	79	79	79	79	79	79	79	79	79
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	0	686	459	16	68	0	0	1	1	1	0	0
Maria a (Missa)	1-!4			4-:- 0			A: 4			A! C		
	lajor1			Major2			Minor1	700		/linor2	10:5	
Conflicting Flow All	68	0	0	1145	0	0	786	786	686	1017	1245	68
Stage 1	-	-	-	-	-	-	686	686	-	100	100	-
Stage 2	-	-	-	-	-	-	100	100	-	917	1145	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	> <b>-</b> >	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1546	.=	-	618	-	(. <del>=</del> .)	312	326	451	218	176	1001
Stage 1	-	-	-	-	-	-	441	451	-	911	816	-
Stage 2	-	.=.	-		-	-,	911	816	-	329	277	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1546	-	-	618	-	1 -	305	317	451	212	171	1001
Mov Cap-2 Maneuver	-	-	-	-	-	-	305	317	-	212	171	-
Stage 1	-	-	-	-	-	-	441	451	-	911	794	-
Stage 2	-	-	-	-	-	-	886	794	-	327	277	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			2.1			14.7			22.1		
HCM LOS	U			۷. ۱			B			C		
TOW LOO							U			U		
		IDI (	E51	EDT	<b>ED D</b>	14/51	VAIDT	14/55	ODL 4			
Minor Lane/Major Mvmt	<u> </u>	VBLn1	EBL	EBT	EBR	WBL	WBT	WBR :				
Capacity (veh/h)		372	1546	-	-	618	-	-	212			
HCM Lane V/C Ratio		0.007	-	-	-	0.027	-		0.006			
HCM Control Delay (s)		14.7	0	-	-	11	0	-				
HCM Lane LOS		В	Α	-	-	В	Α	-	С			
HCM 95th %tile Q(veh)		0	0	-	-	0.1	-	-	0			

Intersection							
Int Delay, s/veh	2.7						
<u> </u>		EDD	QET	QED.	NI\A/I	NI\A/T	
Movement Configurations	EBL	EBR	SET	SER	NWL	NWT	
Lane Configurations Traffic Vol, veh/h	<b>1</b> 03	16	<b>1</b> → 476	2	1	<b>41</b> 2	
Future Vol, veh/h	103	16	476	2	1	412	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	Stop -	None	-		-	None	
Storage Length	0	0	_	-	-	-	
Veh in Median Storage		-	0	-	_	0	
Grade, %	0	_	0	_	_	0	
Peak Hour Factor	94	94	94	94	94	94	
Heavy Vehicles, %	0	0	1	0	0	1	
Mvmt Flow	110	17	506	2	1	438	
Maian/Minar	Alm c A		Male of		Anic C		
	/linor1		Major1		Major2		
Conflicting Flow All	947	507	0	0	508	0	
Stage 1	507	-	-	-	-	-	
Stage 2	440	-	-	-	-	-	
Critical Hdwy	6.4	6.2	_	-	4.1	-	
Critical Hdwy Stg 1	5.4	-	-	-	-	-	
Critical Hdwy Stg 2	5.4	- 2 2	-	-	2.2	-	
Follow-up Hdwy	3.5 292	3.3 570	-	-	1067	-	
Pot Cap-1 Maneuver Stage 1	609	5/0	-	-	1007	( <b>=</b> )	
Stage 1	653	-	-	-		_	
Platoon blocked, %	000			-	-	_	
Mov Cap-1 Maneuver	292	570	-	-	1067	-	
Mov Cap-1 Maneuver	292	570	-	-	1007	-	
Stage 1	609		-	-		_	
Stage 1 Stage 2	652	-	-	-	-	-	
Staye Z	032	_	_	<u>-</u>	_	_	
Approach	EB		SE		NW		
HCM Control Delay, s	22.8		0		0		
HCM LOS	С						
Minor Lane/Major Mvm	t	NWL	NWT	EBLn1 E	EBLn2	SET	SER
Capacity (veh/h)		1067			570		
HCM Lane V/C Ratio		0.001		0.375	0.03	_	<u>-</u>
HCM Control Delay (s)		8.4	0	24.5	11.5	_	_
HCM Lane LOS		A	A	C	В	_	_
HCM 95th %tile Q(veh)		0	-	1.7	0.1	_	-
3111 3311 70110 4(1011)					7.1		

Intersection												
Int Delay, s/veh	0.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	LUL	4	LDIN	TTDL	4	TIDIN	HUL	4	אטוז	ODL	₩	ODIN
Traffic Vol, veh/h	1	458	33	13	400	1	13	0	6	1	1	0
Future Vol, veh/h	1	458	33	13	400	1	13	0	6	1	1	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	_	-	None	_	-	None	-	-	None		_	None
Storage Length	-	-	-	_	-	-	_	_	-	-	-	-
Veh in Median Storage,	# -	0	_	_	0	_	_	0	_	_	0	_
Grade, %	_	0	-	-	0	-	-	0	-	-	0	_
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	1	482	35	14	421	1	14	0	6	1	1	0
Major/Minor N	1ajor1		1	Major2		ı	Minor1		N	Minor2		
Conflicting Flow All	422	0	0	517	0	0	952	952	500	955	969	422
Stage 1	-	-	_	-	_	_	502	502	-	450	450	-
Stage 2	-	-	-	-	-	-	450	450	-	505	519	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1148	-	-	1059	-	-	241	261	575	240	256	636
Stage 1	-	-	-	-	-	-	555	545	-	592	575	-
Stage 2	-	-	-	-	-		592	575	-	553	536	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1148	-	-	1059	-	-	237	256	575	234	251	636
Mov Cap-2 Maneuver	-	-	-	-	-	-	237	256	-	234	251	-
Stage 1	-	-	-	-	-	-	554	544	-	591	565	-
Stage 2	-	-	-	-	-	-	581	565	-	546	535	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.3			18.3			20		
HCM LOS							С			C		
Minor Lane/Major Mvmt		NBLn1	EBL	EBT	EBR	WBL	WBT	WBR:	SBI n1			
Capacity (veh/h)		291	1148	-		1059	-	-	242			
HCM Lane V/C Ratio		0.069		<u> </u>		0.013	<u> </u>		0.009			
HCM Control Delay (s)		18.3	8.1	0	_	8.4	0	_	20			
HCM Lane LOS		C	Α	A	-	Α	A	_	C			
HCM 95th %tile Q(veh)		0.2	0	-	_	0	-	_	0			

Int Delay, s/veh   0.5   SBT   SBR   WBL   WBT   WBR   NBL   NBT   NBR   SBL   SBT   SBR   SBR
Movement         EBL         EBT         EBR         WBL         WBT         WBR         NBL         NBT         NBR         SBL         SBT         SBR           Lane Configurations         Image: Configuration of the co
Lane Configurations         Image: Configuration of the property of the proper
Traffic Vol, veh/h         1         458         33         13         400         1         13         0         6         1         1         0           Future Vol, veh/h         1         458         33         13         400         1         13         0         6         1         1         0           Conflicting Peds, #/hr         0 <t< td=""></t<>
Future Vol, veh/h         1         458         33         13         400         1         13         0         6         1         1         0           Conflicting Peds, #/hr         0 </td
Conflicting Peds, #/hr         0
Sign Control         Free         Stop         O         None         -         -         None         -         -         None         -         -         None         -
RT Channelized       -       -       None       -       -       None       -       -       None       -       -       None         Storage Length       -       -       200       -
Storage Length       -       -       200       -
Veh in Median Storage, #       -       0       -       -       0 </td
Grade, %       -       0       -       -       0       -       -       0       -       -       0       -       -       0       -       -       0       -       -       0       -       0       -       0       -       0       -       0       -       0       0       -       0       0       -       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0         Heavy Vehicles, %       0       1       0       0       1       0       0       0       0       0       0       0       0
Peak Hour Factor         95
Heavy Vehicles, % 0 1 0 0 1 0 0 0 0 0 0
•
Majay/Minay Majay0 Minay0 Minay0
Major/Minor Major1 Major2 Minor1 Minor2
Conflicting Flow All 422 0 0 517 0 0 934 934 482 955 969 422
Stage 1 484 484 - 450 450 - Stage 2 450 450 - 505 519 -
Critical Hdwy Stg 1 4.1 7.1 6.5 6.2 7.1 6.5 6.2 Critical Hdwy Stg 1 6.1 5.5 - 6.1 5.5 -
0.111.111.1.01.0
Follow-up Hdwy 2.2 2.2 3.5 4 3.3 3.5 4 3.3 Pot Cap-1 Maneuver 1148 1059 248 268 588 240 256 636
Stage 1 568 555 - 592 575 -
Stage 2 592 575 - 553 536 -
Platoon blocked, %
Mov Cap-1 Maneuver 1148 1059 244 263 588 234 251 636
Mov Cap-1 Maneuver 244 263 - 234 251 -
Stage 1 567 554 - 591 565 -
Stage 2 581 565 - 547 535 -
5.0g5 L 500 541 500
Approach EB WB NB SB
HCM Control Delay, s 0 0.3 17.9 20
HCM LOS C C
Minor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1
Capacity (veh/h) 299 1148 1059 242
HCM Lane V/C Ratio 0.067 0.001 0.013 0.009
HCM Control Delay (s) 17.9 8.1 0 - 8.4 0 - 20
HCM Lane LOS C A A - A A - C
HCM 95th %tile Q(veh) 0.2 0 0 0

# Appendix E Future Year LOS Calculations



Intersection	<u> </u>					
Int Delay, s/veh	0.1					
Movement	EBL	EBR	SET	SER	NWL	NWT
Lane Configurations	*	7	ĵ.			4
Traffic Vol, veh/h	1	4	1099	1	0	73
Future Vol, veh/h	1	4	1099	1	0	73
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storag	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	0	0	1	0	0	1
Mvmt Flow	1	5	1263	1	0	84
NA : (NA:	NA:				4 : 0	
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	1348	1264	0	0	1264	0
Stage 1	1264	-	-	-	-	-
Stage 2	84	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	168	209	-	-	557	-
Stage 1	268	-	-	-	-	-
Stage 2	944	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	168	209	-	-	557	-
Mov Cap-2 Maneuver		-	-	-	-	-
Stage 1	268	-	-	-	-	-
Stage 2	944	_	_	_	_	_
215.30 2	3.7					
Approach	EB		SE		NW	
HCM Control Delay, s			0		0	
HCM LOS	С					
Minor Lane/Major Mvr	nt	NWL	NWT	EBLn1 l	EBLn2	SET
Capacity (veh/h)		557	-	168	209	-
HCM Lane V/C Ratio		-		0.007		<u>-</u>
HCM Control Delay (s	1	0	_	26.6	22.6	<u>-</u> -
HCM Lane LOS	7	A		20.0 D	22.0 C	-
HCM 95th %tile Q(veh	2)	0	_	0	0.1	
	1)	U	-	U	U. I	-

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	809	294	2	73	0	0	1	1	1	0	0
Future Vol, veh/h	0	809	294	2	73	0	0	1	1	1	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	79	79	79	79	79	79	79	79	79	79	79	79
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	0	1024	372	3	92	0	0	1	1	1	0	0
Major/Minor M	/lajor1			Major2		ı	Minor1		N	Minor2		
Conflicting Flow All	92	0	0	1396	0	0	1308	1308	1210	1309	1494	92
Stage 1	-	-	_	-	-	-	1210	1210	-	98	98	-
Stage 2	_	_	_	_	_	_	98	98	<u>-</u>	1211	1396	_
Critical Hdwy	4.1	_	_	4.1	_	_	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	_	_	-	_	_	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	1-1	_	-	_	-	6.1	5.5	_	6.1	5.5	_
Follow-up Hdwy	2.2	-	_	2.2	-	_	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1515	-	_	496	-	-	138	161	225	138	124	971
Stage 1	-	-	_	-	-	-	225	258	-	913	818	-
Stage 2	-	-	-	-	-	-	913	818	-	225	210	-
Platoon blocked, %		_	_		_	_	1 - 1					
Mov Cap-1 Maneuver	1515	_	_	496	_	-	137	160	225	136	123	971
Mov Cap-2 Maneuver	-	_	_	-	_	_	137	160	-	136	123	-
Stage 1	-	_	-	-	_	_	225	258	-	913	813	_
Stage 2	_	_	_	_	_	_	908	813	_	223	210	_
<b>-g</b>											,	
Annroach	EB			WB			NB			SB		
Approach												
HCM Control Delay, s	0			0.3			24.5			31.7		
HCM LOS							С			D		
Minor Lane/Major Mvmt	t N	VBLn1	EBL	EBT	EBR	WBL	WBT	WBR S				
Capacity (veh/h)		187	1515	-	-	496	-	-	136			
HCM Lane V/C Ratio		0.014	-	-		0.005	-		0.009			
HCM Control Delay (s)		24.5	0	-	-	12.3	0	-				
HCM Lane LOS		С	Α	-	-	В	Α	-	D			
HCM 95th %tile Q(veh)		0	0	-	-	0	-	-	0			

Intersection						
Int Delay, s/veh	2.7					
Movement	EBL	EBR	SET	SER	NWL	NWT
Lane Configurations	*	7	1,			4
Traffic Vol, veh/h	79	13	657	1	0	569
Future Vol, veh/h	79	13	657	1	0	569
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	_	-	_	-
Veh in Median Storage		-	0	_	_	0
Grade, %	, " 0	_	0	_	_	0
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	0	94	1	0	94	1
Mvmt Flow	84	14	699	1	0	605
IVIVIIIL FIOW	04	14	099		U	000
Major/Minor I	Minor1		Major1		Major2	
Conflicting Flow All	1305	700	0	0	700	0
Stage 1	700	-	_	-	-	-
Stage 2	605	_	-	_	_	_
Critical Hdwy	6.4	6.2	_	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	_	-	-
Critical Hdwy Stg 2	5.4	_	_	_	_	=/
Follow-up Hdwy	3.5	3.3	_	_	2.2	_
Pot Cap-1 Maneuver	178	443	_	_	906	_
Stage 1	496	-	_	_	-	_
Stage 2	549					_
Platoon blocked, %	543		_	_		_
· ·	178	443			906	-
Mov Cap-1 Maneuver			-	-		
Mov Cap-2 Maneuver	178	-	-	-	-	-
Stage 1	496	-	-	-	-	
Stage 2	549	-	-	-	-	-
Approach	EB		SE		NW	
HCM Control Delay, s	38		0		0	
HCM LOS	E		J			
TIOWI LOO						
Minor Lane/Major Mvm	ıt	NWL	NWT	EBLn1 l	EBLn2	SET
Capacity (veh/h)		906	-	178	443	-
HCM Lane V/C Ratio		-	-	0.472	0.031	-
HCM Control Delay (s)		0	-	42.1	13.4	-
HCM Lane LOS		Α	_	Е	В	-
HCM 95th %tile Q(veh)		0	-	2.3	0.1	-

Intersection												
Int Delay, s/veh	0.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	643	26	11	559	2	10	0	4	1	1	0
Future Vol, veh/h	1	643	26	11	559	2	10	0	4	1	1	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
•	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	_	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	1	677	27	12	588	2	11	0	4	1	1	0
Major/Minor M	ajor1		1	Major2		N	Minor1		N	Minor2		
Conflicting Flow All	590	0	0	704	0	0	1307	1307	691	1308	1319	589
Stage 1	-	-	-	-	-	-	693	693	-	613	613	-
Stage 2	-	-	-	-	-	-	614	614	-	695	706	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	1-1	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	995	-	-	903	-		138	161	448	138	158	512
Stage 1	-	-	-	-	-	-	437	448	-	483	486	-
Stage 2	-	-	-	-	-		483	486	-	436	442	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	995	-	-	903	-	-	135	157	448	134	155	512
Mov Cap-2 Maneuver	-	-	-	-	-	-	135	157	-	134	155	-
Stage 1	-	-	-	-	-	-	436	447	-	482	476	-
Stage 2	-	-	-	-	-	-	472	476	-	431	441	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.2			28.3			30.4		
HCM LOS							D			D		
Minor Lane/Major Mvmt		NBLn1	EBL	EBT	EBR	WBL	WBT	WBR:	SBL <sub>n1</sub>			
Capacity (veh/h)		169	995	-	-	903	-	-	144			
HCM Lane V/C Ratio		0.087	0.001	-	-	0.013	-	-	0.015			
HCM Control Delay (s)		28.3	8.6	0	-	9	0	-	30.4			
HCM Lane LOS		D	Α	Α	-	Α	Α	-	D			
HCM 95th %tile Q(veh)		0.3	0	-	-	0	-	-	0			

# Appendix F Future Year Plus Project LOS Calculations



Intersection						
Int Delay, s/veh	0.2					
IIII Delay, 8/VeII						
Movement	EBL	EBR	SET	SER	NWL	NWT
Lane Configurations	*	7	1→			र्भ
Traffic Vol, veh/h	2	5	1168	8	2	73
Future Vol, veh/h	2	5	1168	8	2	73
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	_	0	_	-	0
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	0	0	1	0	0	1
Mvmt Flow	2	6	1343	9	2	84
IVIVIIICI IOVV		U	1040	9		UT
Major/Minor	Minor1	N	//ajor1	N	Major2	
Conflicting Flow All	1436	1348	0	0	1352	0
Stage 1	1348	-	-	-	-	-
Stage 2	88	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	_	_	_	_	_
Follow-up Hdwy	3.5	3.3	_	-	2.2	-
Pot Cap-1 Maneuver	149	186	_	_	516	-
Stage 1	244	-	_	_	-	_
Stage 2	940	_	_	_	_	_
Platoon blocked, %	3 10		_	_		_
Mov Cap-1 Maneuver	148	186			516	
Mov Cap-1 Maneuver	148	100	-	_	310	
Stage 1	244	-	-	-	-	-
_		=	-	=	-	-
Stage 2	936	-	-	-	-	-
Approach	EB		SE		NW	
HCM Control Delay, s	26.3		0		0.3	
HCM LOS	D				5.5	
Minor Lane/Major Mvn	nt	NWL	NWT	EBLn1 [		SET
Capacity (veh/h)		516	-		186	-
HCM Lane V/C Ratio		0.004	-	0.016	0.031	-
HCM Control Delay (s)		12	0	29.7	25	-
HCM Lane LOS		В	Α	D	D	-
HCM 95th %tile Q(veh	)	0	-	0	0.1	-
-						

Intersection												
Int Delay, s/veh	0.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	810	363	13	75	0	0	1	1	1	0	0
Future Vol, veh/h	0	810	363	13	75	0	0	1	1	1	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	79	79	79	79	79	79	79	79	79	79	79	79
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	0	1025	459	16	95	0	0	1	1	1	0	0
Major/Minor N	/lajor1			Major2			Minor1		N	/linor2		
Conflicting Flow All	95	0	0	1484	0	0	1382	1382	1255	1383	1611	95
Stage 1	-	-	-	-	-	-	1255	1255	-	127	127	-
Stage 2	-	-	-	-	-	-	127	127	-	1256	1484	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-/	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1512	-	-	459	-	-	122	145	211	122	105	967
Stage 1	-	-	-	-	-	-	212	245	-	882	795	-
Stage 2	-	-	-	-	-	-	882	795	-	212	190	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1512	-	-	459	-	-	119	140	211	117	101	967
Mov Cap-2 Maneuver	-	-	-	-	-	-	119	140	-	117	101	-
Stage 1	-	-	-	-	-	-	212	245	-	882	766	-
Stage 2	-	-	-	-	-	-	849	766	-	210	190	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			1.9			26.8			36.1		
HCM LOS							D			Е		
Minor Lane/Major Mvm	t N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1			
Capacity (veh/h)		168	1512	-	-	459	_	-	117			
HCM Lane V/C Ratio		0.015	-	-	-	0.036	-	-	0.011			
HCM Control Delay (s)		26.8	0	-	-	13.1	0	_	36.1			
HCM Lane LOS		D	A	_	_	В	A	_	E			
HCM 95th %tile Q(veh)		0	0	-	-	0.1	_	-	0			

Intersection												
Int Delay, s/veh	0.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	7		4			4			4	
Traffic Vol, veh/h	0	810	363	13	75	0	0	1	1	1	0	0
Future Vol, veh/h	0	810	363	13	75	0	0	1	1	1	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	200	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	_	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	79	79	79	79	79	79	79	79	79	79	79	79
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	0	1025	459	16	95	0	0	1	1	1	0	0
Major/Minor M	lajor1		1	Major2			Minor1		N	/linor2		
Conflicting Flow All	95	0	0	1484	0	0	1152	1152	1025	1383	1611	95
Stage 1	-	-	-	-	-	-	1025	1025	_	127	127	_
Stage 2	-	-	-	-	-	-	127	127	-	1256	1484	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	_	-	_	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	_	_	_	_	-/	6.1	5.5	_	6.1	5.5	_
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
	1512	-	_	459	-	-	176	199	288	122	105	967
Stage 1	-	-	-	-	-	-	286	315	-	882	795	-
Stage 2	.=:	-	_	-	-	-,	882	795	-	212	190	_
Platoon blocked, %		-	-		-	-						
	1512	_	_	459	_	-	171	192	288	117	101	967
Mov Cap-2 Maneuver	-	_	_	-	_	_	171	192	-	117	101	-
Stage 1	_	-	_	-	-	-	286	315	-	882	766	_
Stage 2	_	_	_	_	_	_	849	766	_	210	190	_
							3.3	. 55		_,,	.00	
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			1.9			20.8			36.1		
HCM LOS				1.0			C			E		
Minor Lane/Major Mvmt	N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR 9	SBLn1			
Capacity (veh/h)		230	1512			459		-	117			
HCM Lane V/C Ratio		0.011	-	<u>-</u>		0.036	-		0.011			
HCM Control Delay (s)		20.8	0	_	_	13.1	0	_	36.1			
HCM Lane LOS		20.0 C	A	_	_	13.1 B	A	_	50.1 E			
HCM 95th %tile Q(veh)		0	0		_	0.1	-	_	0			
HOW JOHN JOHN Q(VEII)		U	U		-	U. I		-	U			

Intersection							
Int Delay, s/veh	4.3						
		EDE	OFT	OFF	N IVA/I	N IV A /T	
Movement	EBL	EBR	SET	SER	NWL	NWT	
Lane Configurations	100	7	<b>^</b>	_		4	
Traffic Vol, veh/h	103	16	664	2	1	572	
Future Vol, veh/h	103	16	664	2	1	572	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None			
Storage Length	0	0	-	-	-	-	
Veh in Median Storage,		-	0	-	-	0	
Grade, %	0	-	0	-	-	0	
Peak Hour Factor	94	94	94	94	94	94	
Heavy Vehicles, %	0	0	1	0	0	1	
Mvmt Flow	110	17	706	2	1	609	
Major/Minor N	1inor1	_ N	Major1	_ N	Major2		ĺ
Conflicting Flow All	1318	707	0	0	708	0	
Stage 1	707	707		U	100		
•	611		-	-	-	-	
Stage 2		6.0	-	-	1.1	-	
Critical Hdwy	6.4	6.2	-	-	4.1	-	
Critical Hdwy Stg 1	5.4	-	-	-	-	-	
Critical Hdwy Stg 2	5.4	-	-	-	-	=	
Follow-up Hdwy	3.5	3.3	-	-	2.2	-	
Pot Cap-1 Maneuver	175	439	-	-	900	-	
Stage 1	493	-	-	-	-	-	
Stage 2	546	-	-	-	-	-	
Platoon blocked, %			-	-		_	
Mov Cap-1 Maneuver	175	439	-	-	900	-	
Mov Cap-2 Maneuver	175	-	-	-	-	-	
Stage 1	493	-	-	-	-	-	
Stage 2	545	-	-	-	-	-	
Approach	EB		SE		NW		
	49.3		0		0		
HCM LOS			U		U		
HCM LOS	E						
Minor Lane/Major Mvmt		NWL	NWT	EBLn1 E	EBL <sub>n2</sub>	SET	
Capacity (veh/h)		900	-		439	_	
HCM Lane V/C Ratio		0.001	_	0.626		-	
HCM Control Delay (s)		9	0	54.9	13.5	-	
HCM Lane LOS		A	A	F	В	-	4
HCM 95th %tile Q(veh)		0	-	3.5	0.1	-	
7000 0(1011)				3.0	<b>J</b> .,		

Intersection												
Int Delay, s/veh	0.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			44>			4			4	
Traffic Vol, veh/h	1	646	33	13	560	1	13	0	6	1	1	0
Future Vol, veh/h	1	646	33	13	560	1	13	0	6	1	1	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
•	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	_	0	_	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	1	680	35	14	589	1	14	0	6	1	1	0
Major/Minor M	ajor1			Major2		N	Minor1			/linor2		
Conflicting Flow All	590	0	0	715	0	0	1318	1318	698	1321	1335	590
Stage 1	-	-	-	-	-	-	700	700	-	618	618	-
Stage 2	-	-	-	-	-	-	618	618	-	703	717	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	: <b>-</b> 1	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	995	-	-	895	-	( <b>.=</b> .)	136	159	444	135	155	511
Stage 1	-	-	-	-	-	-	433	444	-	480	484	-
Stage 2	.=.	-	-	-	-	-	480	484	-	431	437	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	995	-	-	895	-	-	133	155	444	131	151	511
Mov Cap-2 Maneuver	-	-	-	-	-	-	133	155	-	131	151	-
Stage 1	-	-	-	-	-	-	432	443	-	479	473	-
Stage 2	-	-	-	-	-	-	468	473	-	424	436	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.2			28.8			31.1		
HCM LOS							D			D		
Minor Lane/Major Mvmt		NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBL <sub>n1</sub>			
Capacity (veh/h)		171	995	-	-	895	-	_	140			
HCM Lane V/C Ratio		0.117	0.001	-	-	0.015	-	-	0.015			
HCM Control Delay (s)		28.8	8.6	0	-	9.1	0	-	31.1			
HCM Lane LOS		D	Α	Α	-	Α	Α	-	D			
HCM 95th %tile Q(veh)		0.4	0	-	-	0	-	-	0			

Intersection												
Int Delay, s/veh	0.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	7		4			4			4	
Traffic Vol, veh/h	1	646	33	13	560	1	13	0	6	1	1	0
Future Vol, veh/h	1	646	33	13	560	1	13	0	6	1	1	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	200	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	1	680	35	14	589	1	14	0	6	1	1	0
Major/Minor N	1ajor1	Major					Minor1		N	/linor2		
Conflicting Flow All	590	0	0	715	0	0	1300	1300	680	1321	1335	590
Stage 1	-	-	-	-	-	-	682	682	-	618	618	-
Stage 2	-	-	-	-	-	-	618	618	-	703	717	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-		-	-	_	-	6.1	5.5	-	6.1	5.5	_
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	995	-	_	895	-	-	140	163	454	135	155	511
Stage 1	-	-	-	-	-	-	443	453	-	480	484	-
Stage 2	-	-	-	-	-	-	480	484	-	431	437	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	995	_	-	895	_	_	137	159	454	131	151	511
Mov Cap-2 Maneuver	-	-	-	-	-	-	137	159	-	131	151	-
Stage 1	-	_	_	_	_	_	442	452	_	479	473	_
Stage 2	-	-	-	_	_	-	468	473	-	424	436	-
							, 33	,, 3				
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.2			28.1			31.1		
HCM LOS							D			D		
Minor Lane/Major Mvmt		NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1			
Capacity (veh/h)		176	995	_	-	895	_	_	140			
HCM Lane V/C Ratio		0.114		_	_	0.015	_	_	0.015			
HCM Control Delay (s)		28.1	8.6	0	-	9.1	0	-	31.1			
HCM Lane LOS		D	A	A	_	A	A	_	D			
HCM 95th %tile Q(veh)		0.4	0	-	_	0	-	_	0			
		J. 1										

### APPLICATION FOR PERMIT TO APPROPRIATE THE PUBLIC WATERS OF THE STATE OF NEVADA

THIS SPACE FOR C	DFFICE USE ONLY NOV 1 8 2021	i
Date of Filing in State Engineer's Office		
Returned to applicant for correction		
Corrected Application filed	Map filed MAR_ 4_2021_UNDER_S	20473
The applicant Washoe County and City of Reno as to 50% Ea	ach	
1001 F NV 4 G	of Reno	
Street Address or P.O. Box	City or Town	
Nevada, 89512	vbehmaram@washoecounty.us	
State and ZIP Code hereby make(s) application for permission to appropriate t	E-mail Address the public waters of the State of Nevada	as hereinafter stated
	•	
(If applicant is a corporation, give date and place of inc	corporation; if a copartnership or associ	ation, give names of
members.)		
		• ,
<b></b>		-
Grace Lake and Tributaries all	being tributary to Browns Creek	••
	he stream, lake, underground, spring or other sources.	<del></del>
	0.0 AFA (net of 48.3 AFA returned to di	rainage)
	n rate in cubic feet per second (CFS) AND duty in acre-fee	
(a) If stored in a reservoir give the number of acre-feet	9.2 (up to 3 MG of temporary tank stora	ge)
3. The water is to be used for Recreational (Snowmaking		
3. The water is to be used for	ng, commercial, domestic or other use. Must be limited to	one major use.
4. If use is for:		
(a) Irrigation, state number of acres to be irrigated		
(b) Stockwater, state number and kind of animals		
(c) Other use (describe fully in No. 12)		
(d) Power:	· · · · · · · · · · · · · · · · · · ·	
(1) Horsepower developed  (2) Point of return of yester to stream		

5. The water is to be diverted from its source at the following point: (Describe as being within a 40-acre subdivision of public survey, and by course and distance to a found section corner. If on unsurveyed land, it should be so stated.)

A concrete sump within the NW 1/4 of the SE 1/4 of Section 17, T17N, R19 E, MDM, or at a point from which the South 1/4 corner of said Section 17 bears S 06°00'58" W, 1949.48' feet, more or less. Please refer to map on file with Permit No. 90473.

6. Place of use: (Describe by legal subdivision. If on unsurveyed land, it should be so stated)

Washoe County Assessor's Parcel No. 048-050-03, being portions of the SW 1/4, and portions of the W 1/2 of the SE 1/4 of Section 17, T17N, R19E, MDM. Please refer to map on file with Permit No. 90473.

7. Use will begin about October 1st	and end about May	1st of each year.
8. Description of proposed works. (Under the specifications of your diversion or storage drilled well with a pump and motor, etc.)		
No new works are proposed; all infrastructur	re is associated with Decreed Pe	rmit V02748 / Permit No. 90473
9. Estimated cost of works: n/a		
10. Estimated time required to construct work		
11. Patient delice and delice a lead		ell is complete, describe works.)
11. Estimated time required to complete the		·
12. Provide a detailed description of the prop provide a detailed description may cause a delay in proce		(use attachments if necessary): (Failure to
The proposed project will store the non-cons Browns Creek, of which 48.3 or 84 percent was anticipated to consist of up to 3 million gas of manmade snow within the Place of Use.	will be returned via snowmelt ru	noff and infiltration to the source. Storage
13. Miscellaneous remarks:		
Evidence of snowmaking non-consumptive u State Engineer that 9.8 percent of diverted w percent of diverted water may be consumptive	ater is consumed, and Nevada P	ermit No. 85945 requires that only 20
permits@robisoneng.com	Nathan Earl Robison	PE, WRS  Type or print name clearly
E-mail Address		
775-852-2251 700	Robison Engineering	gnature, applicant or agent
Phone No. Ext.	- Rootson Engineering	
APPLICATION MUST BE SIGNED BY THE APPLICANT OR AGENT	PO Box 1505  Sparks, NV 89432	Company Name  Street Address or PO Box

City, State, ZIP Code



### THE STATE OF NEVADA

### PERMIT TO APPROPRIATE WATER

Name of Permittee: WASHOE COUNTY AND CITY OF RENO

Source: STREAM (BROWNS CREEK)

Basin: PLEASANT VALLEY

Manner of Use: RECREATIONAL

Period of Use: OCTOBER 1ST THROUGH MAY 1ST

**Priority Date:** 11/18/2021

\*\*\*\*\*

### **APPROVAL OF STATE ENGINEER**

This is to certify that I have examined the foregoing application, and do hereby grant the same, subject to the following limitations and conditions:

This permit is issued subject to existing rights. The amount of water herein granted is only a temporary allowance, and the final water right obtained under this permit will be dependent upon the amount of water actually placed to beneficial use. A suitable measuring device must be installed and measurements of water use kept. The State retains the right to regulate the use of the water herein granted at any and all times.

Monthly records shall be kept of the amount of water pumped from this well and shall be submitted to the State Engineer on a quarterly basis within 15 days after the end of each calendar quarter.

This permit does not extend the permittee the right of ingress and egress on public, private, or corporate lands.

The issuance of this permit does not waive the requirements that the permit holder obtain other permits from State, Federal, and local agencies. The place of use of this permit is limited to that area lying totally within the Pleasant Valley Hydrographic Basin.

The total combined duty of water under Permits 90473 and 91279 shall not exceed 57.5 acre-feet per season.

The consumptive use of water under Permits 90473 and 91279 shall not exceed 9.2 acre-feet per season. A 16% consumptive use factor was assigned to be consistent with the Truckee-River Operating Agreement. The State Engineer reserves the right to modify the terms of this permit if it is found that the consumptive use of water under these permits for snowmaking purposes exceeds this percentage. (Continued on Page 2)

The point of diversion and place of use are as described on the submitted application to support this permit.

The amount of water to be appropriated shall be limited to the amount that can be placed to beneficial use and shall neither exceed a diversion rate of 0.11487 cubic feet per second nor exceed a seasonal duty of 48.3 acre-feet.

Work must be prosecuted with reasonable diligence and proof of completion of work shall be filed on or before:

July 21, 2024

Water must be placed to beneficial use and proof of the application of water to beneficial use shall be filed on or before:

July 21, 2026

Map in support of proof of beneficial use shall be filed on or before:

N/A

IN TESTIMONY WHEREOF, I, ADAM SULLIVAN, P.E.,

State Engineer of Nevada, have hereunto set my hand and the seal of my office, this \_\_\_\_\_ day of <u>December</u>, 2022

State Engineer

### Sky Tavern Ski Area

**Wetland Enhancement Project** 

**Aquatic Resources Delineation Report** 

**Draft December 2021** 

### Sky Tavern Ski Area

### **Wetland Enhancement Project**

### **Aquatic Resources Delineation Report**

### Draft DECEMBER 2021

Prepared for:

Sky Tavern Board of Directors 21130 Mount Rose Highway Reno, Nevada 89511

Submitted to:

U.S. Army Corps of Engineers Sacramento District Reno Regulatory Field Office 300 Booth Street, Room 3050 Reno, Nevada 89509-1361

Prepared by:



Reno: 5890 Mitra Way Reno, Nevada 89523 Phone: (775) 225-5548

### SKY TAVERN SKI AREA WETLAND ENHANCEMENT PROJECT AQUATIC RESOURCE DELINEATION REPORT

### **EXECUTIVE SUMMARY**

This report presents the results of a delineation of wetlands and other waters that would likely be subject to regulation by the United States Army Corps of Engineers (USACE) under Section 404 of the Clean Water Act. The delineation was conducted at the Sky Tavern Ski Area (Sky Tavern) Wetland Enhancement Project (Project) located in Washoe County, Nevada. The Project is bordered to the north by a paved entrance to Sky Tavern, to the east by Bums Gulch Road, to the south by a short dirt access road, and to the west by a small pond and the Project Discovery Leadership upland ropes course at Sky Tavern. There are multiple small buildings or ropes courses within the Project Area. The Project Area encompasses 2.09 acres on land owned by the City of Reno.

Field work for the delineation was conducted by Michael Robison and Daniel Robison, botanist for Robison Wildlife Consulting, LLC (RWC) in August 2021 using the routine on site determination method described in the *U.S. Army Corps of Engineers Wetlands Delineation Manual* (Environmental Laboratory 1987) and, where applicable, in accordance with methods identified in the *Interim Regional Supplement to the Corps of Engineers Wetland Delineation Manual for the Western Mountains, Valleys, and Coast Region* (USACE 2010). Other waters were mapped and delineated in the field in accordance with the methodology presented in *A Guide to Ordinary High Water Mark (OHWM) Delineation for Non-Perennial Streams in the Western Mountains, Valleys, and Coast Region of the United States* (Mersel and Lichvar 2014).

Based on online mapping found at the USFWS National Wetlands Inventory Map program (https://www.fws.gov/wetlands/data/mapper.HTML) analysis, 2.09 acres of potentially jurisdictional wetland are located within the Project Area. Based on Field Analysis there are no wetlands and 0.15 acres of other waters located within the Project Area. All wetland boundaries and jurisdictional determinations presented in this report are preliminary and subject to verification by the USACE, Sacramento District.

### SKY TAVERN SKI AREA WETLAND ENHANCEMENT PROJECT AQUATIC RESOURCE DELINEATION REPORT

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### SKY TAVERN SKI AREA WETLAND ENHANCEMENT PROJECT AQUATIC RESOURCE DELINEATION REPORT

### 1 INTRODUCTION

### 1.1 Project Description

RWC was retained by Robison Engineering to conduct an aquatic resource delineation on a portion of the Sky Tavern property that is approximately 2.09 acres in size. The area consists of a natural drainage with approximately 40 feet of elevation loss from the south to the north end. The soils consist largely of decomposed granite that has eroded into the Project Area. The northern end is a low, gradually sloped area that leads to a culvert under the parking entrance and to a depression west of the Project Area. The depression has been deepened in the past to create a pond that collects water that is lower than the culvert.

The property is owned by the City of Reno and leased long-term by Sky Tavern, home to the non-profit Sky Tavern Junior Ski Program. The City of Reno tasked Sky Tavern with increasing recreational use of the area and improving the facilities. To this end, Sky Tavern is proposing to increase snowmaking capacity, which would allow the area to open sooner, and add additional parking. Water for snowmaking would be stored in the historical location of Grassy Lake. A portion of the area would be dredged, and the equipment building would be removed.

The purpose of this report is to identify and describe aquatic resources that would be affected by the Project. The aquatic resource delineation and this report have been completed and prepared in accordance with the USACE Sacramento District *Minimum Standards for Acceptance of Aquatic Resource Delineation Reports* (USACE 2016).

### 1.2 <u>Contact Information</u>

Contact information for the applicant and agent is provided below.

### Applicant:

Bill Henderson Executive Director/General Manager Sky Tavern 21130 Mount Rose Highway Reno, Nevada 89511 (775) 323-5125

### Agent:

Michael Robison, Principle Manager and Biologist Robison Wildlife Consulting, LLC 5890 Mitra Way Reno, Nevada 89523 (775) 225-5548

### 2 LOCATION

The Project is located at 21130 Mount Rose Highway, Reno, Nevada 89511, on the eastern slope of Mount Rose in Washoe County. The Project is located entirely within Section 17, Township 17 North, Range 19 East (T17N R19E) Mount Diablo Base and Meridian (Project Area). It is bordered to the north by the parking lot, to the east by Bums Gulch Road, to the south by a dirt access road, and to the west by a pond and Project Discovery Rope course. The Project Area encompasses 2.09 acres.

The Project Area can be reached from the City of Reno by traveling 5.4 miles south on Interstate 580, exiting onto Nevada State Route 431/Mount Rose Highway, and traveling approximately 11 miles to Sky Tavern. Figure 2 shows the Project Area, location and access. All supporting maps are included in Appendix A with the exception of the aquatic resource delineation map which is included in Appendix B.

### 3 METHODS

Field work for the delineation was conducted on August 5 and 14, 2021, by Michael Robison and Daniel Robison, botanists for RWC. The relatively small size of the Project Area, and its position within the landscape allowed for the collection of three-parameter data (vegetation, soils, and hydrology) across the Project Area as a whole, rather than at discreet pairs of data points. The delineation was performed in accordance with the *U.S. Army Corps of Engineers Wetlands Delineation Manual* (Environmental Laboratory 1987) and, where applicable, in accordance with methods identified in the *Interim Regional Supplement to the Corps of Engineers Wetland Delineation Manual for the Western Mountains, Valleys, and Coast Region* (USACE 2010). Other waters of the United States were mapped and delineated in the field in accordance with the 2014 A Guide to Ordinary High Water Mark (OHWM) Delineation for Non-Perennial Streams in the Western Mountains, Valleys, and Coast Region of the United States (Mersel and Lichvar 2014).

All Geographic Information System (GIS) data were recorded in the Universal Transverse Mercator (UTM) coordinate system in the North American Datum of 1983 (NAD 83) for Zone 11 North (meters) with a Garmin 64st unit (Garmin).

This report was prepared in accordance with the Sacramento District's *Minimum Standards* for Acceptance of Aquatic Resources Delineation Reports (USACE 2016). The most recent National Wetland Plant List (Lichvar et al. 2016) was used to determine the wetland indicator status of plants observed in the Project Area. The PLANTS database (United States Department of Agriculture [USDA], National Resource Conservation Service [NRCS] 2018) was used for plant nomenclature, except where it conflicted with the nomenclature in the National Wetland Plant List, which was given priority.

### 4 EXISTING CONDITIONS

### 4.1 <u>Landscape Setting and Climate</u>

The Project Area is located at between 7,560 and 7,600 feet above mean sea level (amsl). According to the Western Regional Climate Center (WRCC), the average maximum temperature recorded at the Mount Rose Bowl, Nevada field station, which is located approximately 0.8 mile north of the Project Area, is approximately 75.3 degrees Fahrenheit (°F) in August, and the average temperature is approximately 21.1 °F in February. The average annual precipitation is 6.7 inches and tends to peak in December (WRCC 2016).

### 4.2 <u>Vegetation</u>

Two vegetation communities were identified within the Project Area and described below. All communities are mapped in Figure 3.

### 4.2.1 Sierra Nevada Willow Scrub-Shrub

Approximately 0.41 acres of the Project Area consists of a scrub-shrub plant community of dense stands of Lemmon's willow (Salix lemmonii) interspersed with grasses and barren areas. Other shrub species observed within this plant community included shining willow (Salix lucida), Coyote willow (Salix exigua), gray alder (Alnus incana), and sparse quaking aspen (Populus tremuloides). Forbs observed within this plant community included common yarrow (Achillea millefolium), streambank arnica (Arnica lanceolata), bull thistle (Cirsium vulgare), common pussypaws (Cistanthe monosperma), fringed willowherb (Epilobium ciliatum), common horsetail (Equisetum arvense), racemed groudsmoke (Gayophytum racemosum), western marsh cudweed (Gnaphalium palustre), red-stem stork's bill (Erodium cicutarium), garden babysbreath (Gypsophila scorzonerifolia), seep monkeyflower (Mimulus guttatus), slender cinquefoil (Potentilla gracilis), curly dock (Rumex crispus), common dandelion (Taraxicum officinale), common mullein (Verbascum thapsus), and American speedwell (Veronica americana). Grasses observed within this plant community included rough bentgrass (Agrostis scabra), creeping bentgrass (Agrostis stolonifera), slender wheatgrass (Elymus trachycaulus), Baltic rush (Juncus balticus), toad rush (Juncus bufonius), swordleaf rush (Juncus ensifolius), foxtail barley (Hordeum jubatum), mat muhly (Muhlenbergia richardsonii), timothy (Phleum pratense), Nebraska sedge (Carex nebrascensis), Northwest Territory sedge (Carex utriculata), and Kentucky bluegrass (Poa pratense). Grasses were generally sparse and patchy with no meadow formation. The edges of the community near the paved road had rubber rabbitbrush (Ericameria nauseosa) and other upland plants such as desert Indian paintbrush (Castilleja angustifolia) and cushion buckwheat (*Eriogonum ovalifolium*).

### 4.2.2 Rocky Mountain Subalpine-Montane Riparian Woodland

Aproximately 1.67 acres of the Project Area consists of Rocky Mountain Subalpine-Montane Riparian

Woodland plant community consisting of Quaking aspen, grey alder, Jeffery pine (Pinus jeffreyi), and white fir (Abies concolor). This community occurs in upland areas along the eastern boundary of the Project Area and all of the southern 2/3rds of the Project Area. Forbs observed in this community included common yarrow, mealy goosefoot (Chenopodium incanum), poison hemlock (Conium maculatum), pinnate tansy mustard (Descurainia pinnata), racemed groundsmoke, garden babysbreath, false Solomon's seal (Maianthemum racemosum), woodland pinedrops (Pterospora andromedea), sheep sorrel (Rumex acetosella), and common cocklebur (Xanthium strumarium).

### 4.3 <u>Soils</u>

The entirety of the Project Area, 2.09 acres, is mapped by the NRCS as wetland (Figure 4). The Project Area is composed of two soil types: Tallac very bouldery sandy loam, four to 30 percent slopes; and Graylock-Temo-Rock outcrop complex, 30 to 70 percent slopes.

### 4.3.1 Tallac Sandy Loam Series

The Tallac sandy loams are a series of well-drained soils with a moderately low to moderately high available water capacity. Profiles range from 42 to 65 inches deep to cemented material. These soils do not flood, do not pond, and the depth to the water table is greater than 80 inches. Where mapped, the Tallac and similar soils compose 85 percent of the soil makeup, while other minor components make up the remaining 15 percent. The Tallac series are composed primarily of weathered glaciomarine deposits (NRCS 2018). This soil series is not hydric.

### 4.3.2 Graylock-Temo-Rock Outcrop Complex

The Graylock series of soils are somewhat excessively drained soils with a very low available water capacity. Profiles range from 56 to 66 inches deep to bedrock. These soils do not flood, do not pond, and the depth to the water table is greater than 80 inches. The Graylock series are composed primarily of residuum and colluvium derived from granitic rocks. This soil series is not hydric. The Temo series of soils are excessively drained soils with a very low to moderately low available water capacity. Profiles range from 16 to 60 inches deep to bedrock. These soils do not flood, do not pond, and the depth to the water table is greater than 80 inches. The Temo series are composed primarily of residuum and colluvium derived from granitic rocks. This soil series is not hydric.

Where mapped, the Graylock-Temo-Rock outcrop complex is composed of 50 percent Graylock and similar soils, 25 percent Temo and similar soils, ten percent Rock outcrop, while other minor components make up the remaining 15 percent (NRCS 2018).

### 4.4 Geology

Geologic maps from the United States Geologic Survey (USGS) indicate that the majority of the Project Area is underlain by lake and stream deposits. Lake deposits include gravel, sand, and silt laid down by higher lakes nearly contemporaneous with Lake Lahontan and in post-Lake Lahontan lakes. These deposits are of Pleistocene to Recent age. Small amounts of Tahoe Till, a type of glacial deposit, and water are also mapped within the Project Area (Figure 5) (USGS 1950).

### 4.5 **Hydrology**

The National Hydrography Dataset (NHD) reports that there are no drainages mapped within the Project Area. The nearest mapped water is an ephemeral drainage approximately 1000 feet northeast of the Project Area (Figure 6). NHD flowline data shows this ephemeral drainage connecting with Browns Creek approximately one mile east-southeast from the Project Area. Browns Creek flows into Steamboat Creek, which flows into the Truckee River, the nearest traditional navigable water (TNW), approximately 16 linear miles northeast of the Project Area (USGS 2018).

The National Wetland Inventory (NWI) maps two types of wetlands within the Project Area (Figure 6):

- 1. PSS1B (Palustrine, Scrub-shrub, Broad-Leaved Deciduous, Seasonally Saturated); and
- 2. PFO1C (Palustrine, Forested, Broad-Leaved Deciduous, Seasonally Flooded) (United States Fish and Wildlife Service [USFWS] 1999).

No seasonally flooded wetland areas were observed during field studies.

### 4.6 Aquatic Resources

Aquatic resources within the Project Area consist of one ephemeral stream channel (Channel A). Photographs of the resource are included in Appendix C, OHWM data sheets are included in Appendix D, and Wetland delineation data sheets are included in Appendix E.

### 4.6.1 Channel A

Channel A enters the Project Area from a culvert on the south side of the Project Area and flows north through an incised drainage (not mapped in NHD) until it reaches the north west corner and flows outside of the Project Area in two locations. The channel splits in two. The first place the channel leaves the Project Area leads to the existing pond to the west of the Project Area. The second leads to a culvert that flows under the parking lot entrance and into the old grassy lake area. Water pools in a wider channel before flowing out of a culvert on the north side of the Project Area. After passing into the grassy lake wetland area, the channel passes through another culvert under Mount Rose Highway, where the stream channel re-emerges on the north side of the highway. The stream channel then continues to flow north-northeast for approximately 765 linear feet, where it reaches the ephemeral drainage mapped by the NHD and discussed in Section 4.5. Channel A has bed and bank, as well as OHWM indicators including a break in slope, change in vegetation, and sediment sorting (Appendix C, Photo Plates 2 and 3). Average OHWM width for Channel 1 was 4 feet.

**Table 1. Aquatic Resources within the Project Area** 

Aquatic Resource	Aquatic	<b>Resources Clas</b>	sification	A ava a	Lincou Foot
Name	Cowardin	Location (UTM)*		Acres	Linear Feet
Channel A	R4SB	0252492E 4357943N		0.15	714
Total				0.15	714

<sup>\*</sup>NAD83

### 5 REFERENCES

- Environmental Laboratory. 1987. Corps of Engineers Wetlands Delineation Manual. Technical Report Y-87-1, U.S. Army Engineer Waterways Experiment Station. Vicksburg, Miss.
- Lichvar, R.W., D.L. Banks, W.N. Kirchner, and N.C. Melvin. 2016. The National Wetland Plant List: 2016 wetland ratings. Phytoneuron 2016-30: 1-17. Published 28 April 2016. ISSN 2153 733X.
- Mersel, M.K. and R.W. Lichvar. 2014. A Guide to Ordinary High Water Mark (OHWM)

  Delineation for Non-Perennial Streams in the Western Mountains, Valleys, and

  Coast Region of the United States. ERDC/CRREL TR-14-13. Hanover, NH: Cold

  Regions Research and Engineering Laboratory (CRREL) U.S. Army Engineer Research

  and Development Center.
- Natural Resources Conservation Service (NRCS). 2018. Web Soil Survey. Soil Survey Staff, United States Department of Agriculture. Available online at: http://websoilsurvey.nrcs.usda.gov/. Accessed October 2021.
- United States Army Corps of Engineers (USACE). 2010. Regional Supplement to the Corps of Engineers Wetland Delineation Manual: Western Mountains, Valleys, and Coast Region (Version 2.0), ERDC/EL TR-10-3. Vicksburg, MS: U.S. Army Engineer Research and Development Center.
- \_\_\_\_\_\_. 2016. Minimum Standards for Acceptance of Aquatic Resources Delineation Reports. U.S. Army Corps of Engineers, Sacramento District. Dated January 2016. Available online at: https://www.spk.usace.army.mil/Portals/12/documents/regulatory/jd/minimum
  - standards/Minimum\_Standards\_for\_Delineation\_with\_Template-final.pdf.
    Accessed November 28, 2018.
- United States Department of Agriculture (USDA), NRCS. 2018. The PLANTS Database (http://plants.usda.gov, 27 November 2018). National Plant Data Team, Greensboro, NC 27401-4901 USA
- United States Fish and Wildlife Service (USFWS). 1999. National Wetlands Inventory website.

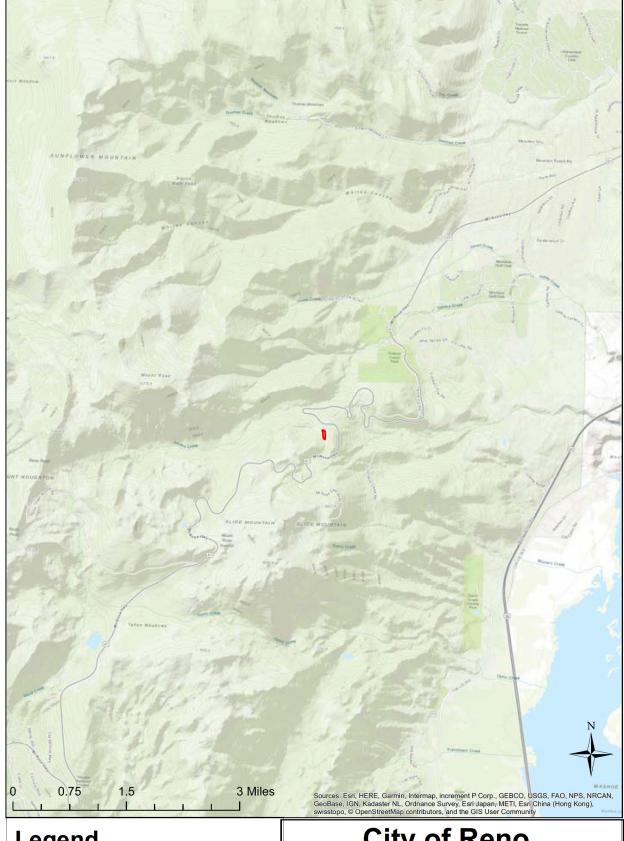
  U.S. Department of the Interior, Fish and Wildlife Service, Washington, D.C. http://www.fws.gov/wetlands/. Accessed November 26, 2018.
- United States Geological Survey (USGS). 1950. Geologic Map and Sections of the Mount Rose Quadrangle, Nevada. Professional Paper 458-A. Reston, Va: U.S. Department of the

Interior.
2018. National Hydrography Geodatabase: The National Map viewer available online at: https://www.usgs.gov/core-science-systems/ngp/national-hydrography.
Accessed November 2018.

Western Regional Climate Center (WRCC). 2016. Historical Climate Information for Mt. Rose Bowl, Nevada 1981-2010 monthly summary. Desert Research Institute. Reno, Nevada. https://wrcc.dri.edu/cgi-bin/cliMAIN.pl?nv5440. Accessed November 2018.

### APPENDIX A

### **Supporting Maps**



Biological Survey Area

### City of Reno

### Sky Tavern Wetland Enhancement Project

Project Location and Access Figure 1

	Date: December 17, 2021	Drawn By: DR
	Project Number: 1110	
	Filename: 1110 Figure1 Location	



Biological Survey Area

### **Vegetation Communities**

Rocky Mountain Subalpine-Montane Riparian Woodland
Sierra Nevada Willow Scrub-Shrub

### City of Reno

### Sky Tavern Wetland Enhancement Project

Vegetation Communities Figure 2

Date: December 17, 2021	Drawn By: DR
Project Number: 1110	
Filename: 1110_Figure2_Vegetation	



Biological Survey Area

### **Soil Association**

Graylock-Temo-Rock outcrop

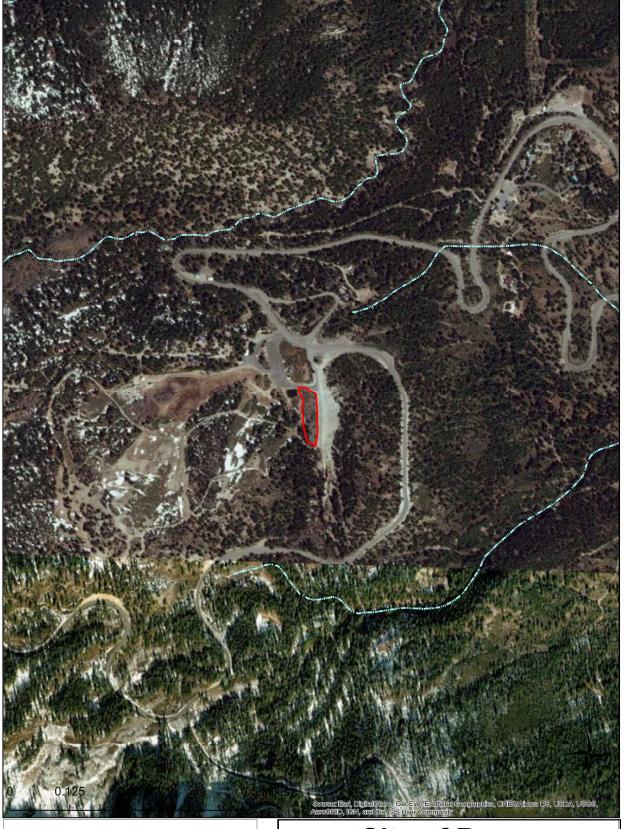
Tallac very bouldery sandy loam

### City of Reno

### Sky Tavern Wetland Enhancement Project

NRCS Soils Figure 3

Date: December 17, 2021	Drawn By: DR
Project Number: 1110	
Filename: 1110_Figure3_Soils	



Biological Survey Area
Intermittent Streams

### **City of Reno**

### Sky Tavern Wetland Enhancement Project

Hydrology Figure 4

Date: December 17, 2021	Drawn By: DR
Project Number: 1110	
Filename: 1110_Figure4_Hydro	

### APPENDIX B

### **Aquatic Resources Delineation Map**



Biological Survey Area
Channel A

### City of Reno

### Sky Tavern Wetland Enhancement Project

Aquatic Resources Delineation Figure 5

Date: December 17, 2021	Drawn By: DR
Project Number: 1110	
Filename: 1110_Figure5_Delineation	

### APPENDIX C

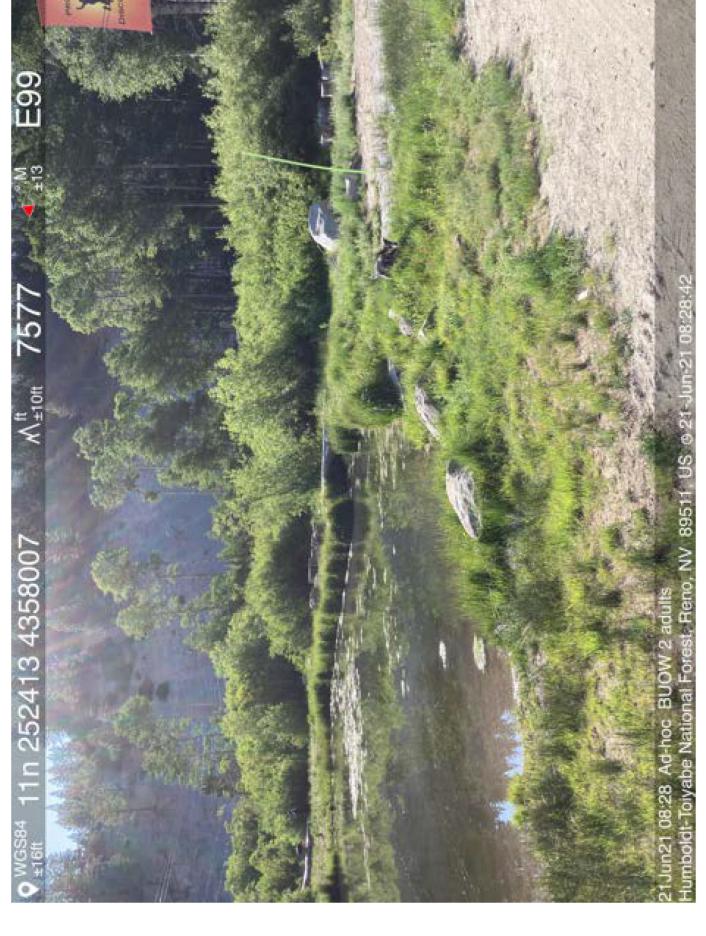
**On-Site Photographs** 

# Sky Tavern Wetland Delineation 2021 – Photo Log



Google Earth view from the North facing South.

Sky Tavern Wetland Project Area 2021 as seen from the West overlooking the pond.



# Channel Bank Cut Observations



Midway Point in the Channel

Southern edge as channel comes out of the culvert.



Channel transition area of upland and willow shrub



Drainage culvert at the northern end of the project area.

## Vegetation Communities





Rocky Mountain Subalpine-Montane Riparian Woodland

Rocky Mountain Subalpine-Montane Riparian Woodland



Sierra Nevada Willow Scrub-Shrub



Sierra Nevada Willow Scrub-Shrub

### Soil Pit Observations



Pit about 4 ft from the stream channel. Approximately 2 ft deep. No ground water Found at the stream depth.



Fine to course sand in the pit. Slight color change in the soil. Little to no oxidation observed.

### APPENDIX D OHWM Data Sheets

OHWM Delineation Cover Sheet Page / of 4
Project: Sky Tavern Wetland Enhancement Date: 8/5/2021
Location: 2H36 Mount Prose Huy Rene NV 8951/ Investigator(s): Muchael Robison
Project Description: The Sky Tavern board of directors proposes to use this area as a wetland enhancement project as mitigation for another project or site,
Describe the river or stream's condition (disturbances, in-stream structures, etc.):
The stream enters the project area at the southern edge through a subset
and exits through a culvert on the north edge and a gate that feeds into a pond just west of the project.
Off-site Information
Remotely sensed image(s) acquired?  Yes No [If yes, attach image(s) to datasheet(s) and indicate approx. locations of transects, OHWM, and any other features of interest on the image(s); describe below] Description:
Hydrologic/hydraulic information acquired?  Yes No [If yes, attach information to datasheet(s) and describe below.] Description:
List and describe any other supporting information received/acquired:
Information for the project was compiled through database searches from the USGS, USFWS wetland Mapper, Google Earth, ESRI and location history from the proponent.
Instructions: Complete one cover sheet and one or more datasheets for each project site. Each datasheet should capture the dominant characteristics of the OHWM along some length of a given stream. Complete enough datasheets to adequately document up- and/or downstream variability in OHWM indicators, stream conditions, etc. Transect locations can be marked on a recent pariel image or their CRS.

coordinates noted on the datasheet.

Datasheet #		OHWM	Delineation D	atasheet	P	age 2 of 4	
Transect (cross-se some distance; labe	ection) drawing: el the OHWM and	(choose a location other features of	interest along th	e transect; include	le an estimate o	f transect length)	
\			Transe	et 10 the	first 232	off from the	
	Asterp side slopes Southern end.						
Change in Vegetation		break in Slope cak in slope	V				
to woody	bro	enk in slope					
upland'	Wh	ere OHWM 15	located				
Alder t Aspen							
Break in Slope at		Sharp (> 60°)   🗵					
Notes/Description:	During His	some it of	the strea	m there is	a conti	wous drop	
in elevation	1 and a	narrow, some	times deep	cut (3 ft)	in the no	-mal slope,	
Sediment Texture	e: Estimate perce	ntages to describe	the general sedi	ment texture abo	ve and below th	e OHWM	
	Clay/Silt <0.05mm	Sand 0.05 – 2mm	Gravel 2mm – 1cm	Cobbles 1 – 10cm	Boulders >10cm	Developed Soil Horizons (Y/N)	
Above OHWM	20	60	20		CHINE !	Y	
Below OHWM	10	30	60			N	
	· · · · · ·		16 co is	Jadima ht	sorting o	and collection	
wall cuts	in the chan	There is , and, Lots o	f woody	debnis is	Sound in t	he channel.	
						below the OHWM	
	Tree (%)	Shrub (%)	Herb (%)	Bare (%)			
Above OHWM	45	15	40				
Below OHWM	5	0	25	70			
Notes/Description	: This seame	est of the 57	tream has	very little	veg etation	within the	
active the	umel. The	re is nearly	100% can	opy cover	from the su	within the mounding trees.	
Other Evidence:	List/describe any	additional field e	vidence and/or li	ines of reasoning	used to suppor	your delineation	
4.11.00.11.200							

OHWM Delineation Cover Sheet	Page <u>3</u> of <u>4</u>
Project: 5Ky Tavery Wetland Enhancement Date: 8/5/2021	Trainers distance and research
Location: 211 30 Mount Rose they, Reno NV89511 Investigator(s): Michael	el Robisón
Project Description:	
Describe the river or stream's condition (disturbances, in-stream structures, etc.):	
Off-site Information	
Remotely sensed image(s) acquired?  Yes No [If yes, attach image(s) to da locations of transects, OHWM, and any other features of interest on the image(s); described to the image (s) interest on the image (s).	stasheet(s) and indicate approx.
, and any other reactives of interest on the image(s), descr	the below] Description:
Hydrologic/hydraulic information acquired?  Yes No [If yes, attach inform	nation to datasheet(s) and describe
below.] Description:	and to damshood by and describe
List and describe any other supporting information received/acquired:	
Instructions: Complete one cover sheet and one or more datasheets for each project site. Each datash	heet should capture the dominant
downstream variability in OHWM indicators, stream conditions, etc. Transect locations can be mark	dequately document up- and/or
ownstream variability in OHWM indicators, stream conditions, etc. Transect locations can be mark pordinates noted on the datasheet.	ed on a recent aerial image or their GPS

Clay/Sill Sulla Start	nsectiong in 3268t
Break in Slope at OHWM: Sharp (> 60°) Moderate (30-60°) Gentle (< 30°) Non Notes/Description: The channel has nearly vertical edges, with some waler cut.  Sediment Texture: Estimate percentages to describe the general sediment texture above and below the OH Clay/Silt Sand Gravel Cobbles Boulders Dec.  Above OHWM 20 50 20  Below OHWM 10 40 50  Notes/Description: In this section a lot of the ecrosion scalement of the compact during high runoff events. The area around the column of the column of the column of the column.	ne areas
Break in Slope at OHWM: Sharp (>60°)   Moderate (30-60°)   Gentle (<30°)   Non Notes/Description: The channel has nearly vertical edges, with some wholes and Gravel Cobbles Boulders  Clay/Silt Sand Gravel Cobbles Boulders  Clay/Silt Sand Gravel Cobbles Boulders  Clay/Silt Sand Gravel 1-10cm >10cm Ho  Above OHWM 20 60 20  Below OHWM 10 40 50  Notes/Description: In this section a lot of the emosion scalement of trapped during high runoff events. The area around the channel of the emosion scalement of	ne carcus HWM
Notes/Description: The channel has nearly vertical edges, with some unclerent.  Sediment Texture: Estimate percentages to describe the general sediment texture above and below the OF Clay/Silt Sand Gravel Cobbles Boulders De <0.05mm 0.05-2mm 2mm-1cm 1-10cm >10cm Ho  Above OHWM 20 50 20  Below OHWM 10 40 50  Notes/Description: In this section a lot of the errosion sedement graph cannot fevents. The area around the channel of the entering the channel of the entering of the entering the channel of the entering of the entering the entering of the entering the entering of the entering the entering the entering of the entering the entering of the entering the entering of the entering the entering the entering of the enterin	areas HWM
Sediment Texture: Estimate percentages to describe the general sediment texture above and below the OF Clay/Silt Sand Gravel Cobbles Boulders De <0.05mm 0.05-2mm 2mm-1cm 1-10cm >10cm Ho  Above OHWM 20 60 20  Below OHWM 10 40 50  Notes/Description: In this section a lot of the errosion sedement graph runoff events. The area around the charge of the errosion of the	HWM
Clay/Silt Sand Gravel Cobbles Boulders De 1-10cm Above OHWM 20 60 20  Below OHWM 10 40 50  Notes/Description: In this section a lot of the errosion scalement g trapped during high runoff events. The area around the characteristics.	HWM eveloped Soil
Above OHWM 20 60 20  Below OHWM 10 40 50  Notes/Description: In this section a lot of the errosion scalement g trapped during high runoff events. The area around the ch	eveloped Son
Below OHWM 10 40 50 Notes/Description: In this section a lot of the errosion sedement of trapped during high runoff events. The area around the	orizons (Y/N)
Notes/Description: In this section a lot of the errosion sedement of trapped during high runoff events. The area around the	
Notes/Description: In this section a lot of the errosion sedement of trapped during high runoff events. The area around the co consists mostly of sand and gravel mix. It is highly draining so	E
Vegetation: Estimate absolute percent cover to describe general vegetation characteristics above and below	)./. 
Tree (%) Shrub (%) Herb (%) Bare (%)	
Above OHWM 15 30 '40 15	
Below OHWM 0 0 30 70 Notes/Description: Some vegetation is observed within the stream in the	
Notes/Description: Some Vegetation is abserved within the stream in the Other Evidence: List/describe any additional field evidence and/or lines of reasoning used to support you	<u> </u>

### APPENDIX E Wetland Delineation Data Sheet

### WETLAND DETERMINATION DATA FORM – Western Mountains, Valleys, and Coast Region

rojecusite: Sky Tavern	City/County: The	1.0 Sampling Date: 8/5/2
pplicant/Owner Sky Tavyn		State: 1/1/ Sampling Point:
westigator(s): M. Robison , D. Robison	Section, Township, Ra	nge:
andform (hillslope, terrace, etc.): Terrace	Local relief (concave,	convex, none): //one Slope (%):
ubregion (LRR): Lat:		
oil Map Unit Name:		NWI classification:
re climatic / hydrologic conditions on the site typical for this time of y		
re Vegetation		Normal Circumstances" present? Yes V No
re Vegetation A/, Soil /V, or Hydrology /V naturally pr		reded, explain any answers in Remarks.)
이 살아보니 있다니? 나를 내용하는 것이 있어? 그는 사람이 없어요? 그는 그 없는 사람이 없다.		
UMMARY OF FINDINGS - Attach site map showing		ocations, transects, important features, etc.
Hydrophytic Vegetation Present? Yes No		
Hydric Soil Present? Yes No	Is the Sampled within a Wetlan	
Wetland Hydrology Present? Yes No		
Remarks: Wetland conditions present will	him the stream	channel, outside of
changel is upload.	222	
EGETATION – Use scientific names of plants.		
Tree Stratum (Plot size:) Absolute  % Cover	Dominant Indicator Species? Status	Dominance Test worksheet:
1. Populus transleides 60	Y FALUP	Number of Dominant Species That Are OBL, FACW, or FAC:  (A)
2 Salix lemmon 5	N FACW	
3 Salix lucida 25	Y FACW	Total Number of Dominant Species Across All Strata: 6 (B)
1 Alous Phone Mana 10	Y FACW	
96	= Total Cover	Percent of Dominant Species That Are OBL, FACW, or FAC: 83 /c (A/B)
Sapling/Shrub Stratum (Plot size:)		Prevalence Index worksheet:
·		Total % Cover of: Multiply by:
2		OBL species 30 x1= 30
3		FACW species >0 x2= 140
4		FAC species 10 x3= 30
5		FACU species65 x4=_260
Herb Stratum (Plqt size:)	_ = Total Cover	UPL species x 5 =
1. Equisation avenue 10	Y FAC	Column Totals: 175 (A) 460 (B)
2 Janus arcticus	N BACW	Prevalence Index = B/A = 2.6
3. Comex repressionsis 25	140 h	Hydrophytic Vegetation Indicators:
. Carex utriculata		X 1 - Rapid Test for Hydrophytic Vegetation
s Agrostis stolaitura 25	Y FACW	X 2 - Dominance Test is >50%
s. Phleum protense 5	- N FACUP	X 3 - Prevalence Index is ≤3.0*
,		4 - Morphological Adaptations' (Provide supporting
B		data in Remarks or on a separate sheet)
9		5 - Wetland Non-Vascular Plants'
10		Problematic Hydrophytic Vegetation¹ (Explain)
11		<sup>1</sup> Indicators of hydric soil and wetland hydrology must be present, unless disturbed or problematic.
	Transference	and proceedings of the processing of the process
75	_= Total Cover	
Woody Vine Stratum (Plot size:)		H. david alla
Woody Vine Stratum (Plot size:)  1		Hydrophytic Vegetation
1		
Woody Vine Stratum (Plot size:)  1		Hydrophytic Vegetation Present? Yes No

PL=Pore Lining, M=Matrix.  Problematic Hydric Soils*:  (A10)  If Material (TF2)  ow Dark Surface (TF12)  plain in Remarks)  ydrophytic vegetation and irology must be present, ribed or problematic.
PL=Pore Lining, M=Matrix.  Problematic Hydric Soils <sup>3</sup> :  (A10)  If Material (TF2)  ow Dark Surface (TF12)  plain in Remarks)  sydrophytic vegetation and  irology must be present,  irbed or problematic.
Problematic Hydric Soils3: (A10) If Material (TF2) ow Dark Surface (TF12) Islain in Remarks) Indicate the problematic of the pr
y with Floodevent
ndicators (2 or more required)
itained Leaves (B9) (MLRA 1, 2 and 4B) e Patterns (B10) ison Water Table (C2) on Visible on Aerial Imagery (CS
phic Position (D2) Aquitard (D3) utral Test (D5) Ant Mounds (D6) (LRR A) save Hummocks (D7)
ent? Yes 🗶 No
a so it we it

### APPENDIX F Plant List

### Species Observed within and in the Vicinity of the Sky Tavern Project Area

Scientific Name	Common Name	
Trees		
Abies concolor	White fir	
Alnus incana	Grey alder	
Pinus jeffreyi	Jeffrey pine	
Populus tremuloides	Quaking aspen	
Salix exigua	Coyote willow	
Salix lemmonii	Lemmon's willow	
Salix lucida	Shining willow	
Shrubs		
Ericameria nauseosa	Rubber rabbitbrush	
Forbs		
Achillea millefolium	Common yarrow	
Arnica lanceolata	Streambank arnica	
Castilleja angustifolia	Desert Indian paintbrush	
Chenopodium incanum	Mealy goosefoot	
Cirsium vulgare <sup>1</sup>	Bull thistle	
Cistanthe monosperma	Common pussypaws	
Conium maculatum	Poison hemlock	
Descurainia pinanta	Pinnate tansy mustard	
Epilobium ciliatum	Fringed willowherb	
Eriogonum ovalifolium	Cushion buckwheat	
Equisetum arvense	Common horsetail	
Gayophytum racemosum	Racemed groundsmoke	
Gnaphalium palustre	Western marsh cudweed	
Erodium cicutarium	Red-stem stork's bill	
Gypsophila scorzonerifolia	Garden babysbreath	
Maianthemum racemosum	False Solomon's seal	
Mimulus guttatus	Seep monkeyflower	
Potentilla gracilis	Slender cinquefoil	
Pterospora andromedea	Woodland pinedrops	
Rumex acetosella	Sheep sorrel	
Rumex crispus	Curly dock	
Taraxicum officinale	Common dandelion	
Verbascum thapsus	Common mullein	
Veronica americana	American speedwell	
Xanthium strumarium	Common cocklebur	
Grass		
Agrostis scabra	Rough bentgrass	
Agrostis stolonifera	Creeping bentgrass	
Carex nebrascensis	Nebraska sedge	
Carex utriculata	Northwest Territory sedge	

### Species Observed within and in the Vicinity of the Sky Tavern Project Area

Scientific Name	Common Name
Elymus trachycaulus	Slender wheatgrass
Juncus arcticus	Baltic rush
Juncus bufonius	Toad rush
Juncus ensifolius	Swordleaf rush
Hordeum jubatum	Foxtail barley
Muhlenbergia richardsonii	Mat muhly
Phleum pratense	Timothy
Poa pratensis	Kentucky bluegrass

<sup>&</sup>lt;sup>1</sup> Nevada noxious weed species